

### HISTORIC PRESERVATION COMMISSION

Marc Elrich
County Executive

Sandra I. Heiler Chairman

Date: November 2, 2020

### **MEMORANDUM**

TO: Hadi Mansouri

Department of Permitting Services

FROM: Michael Kyne

Historic Preservation Section

Maryland-National Capital Park & Planning Commission

SUBJECT: Historic Area Work Permit #907051: Porch Construction

The Montgomery County Historic Preservation Commission (HPC) has reviewed the attached application for a Historic Area Work Permit (HAWP). This application was **Approved** at the March 25, 2020 HPC meeting.

The HPC staff has reviewed and stamped the attached construction drawings.

THE BUILDING PERMIT FOR THIS PROJECT SHALL BE ISSUED CONDITIONAL UPON ADHERENCE TO THE ABOVE APPROVED HAWP CONDITIONS AND MAY REQUIRE APPROVAL BY DPS OR ANOTHER LOCAL OFFICE BEFORE WORK CAN BEGIN.

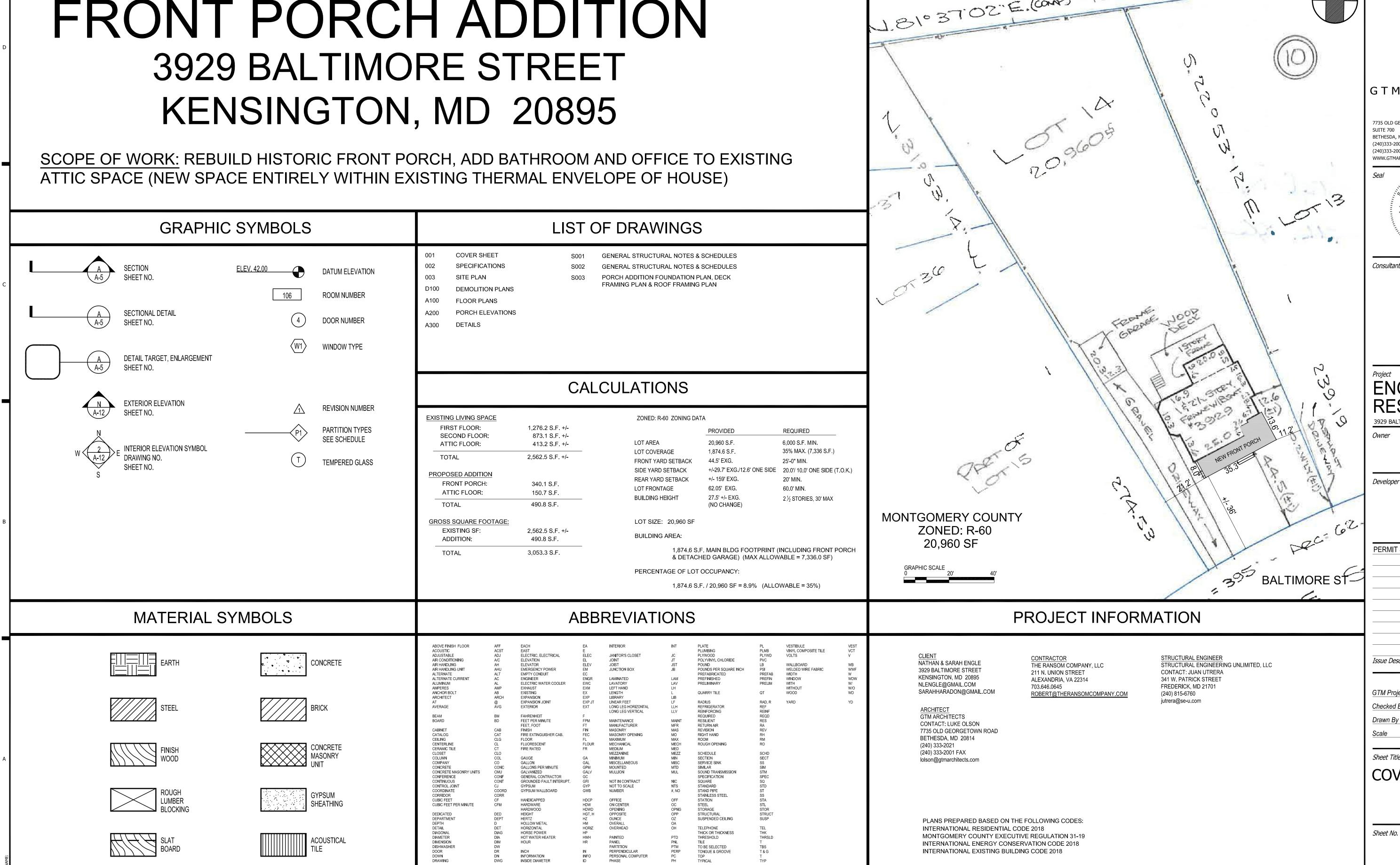
Applicant: Nathan and Sarah Engle (Luke Olson, Architect)

Address: 3929 Baltimore Street, Kensington

This HAWP approval is subject to the general condition that the applicant will obtain all other applicable Montgomery County or local government agency permits. After the issuance of these permits, the applicant must contact this Historic Preservation Office if any changes to the approved plan are made. Once work is complete the applicant will contact Michael Kyne at 301.563.3403 or <a href="michael.kyne@montgomeryplanning.org">michael.kyne@montgomeryplanning.org</a> to schedule a follow-up site visit.



# ENGLE RESIDENCE FRONT PORCH ADDITION



Montgomery County istoric Preservation Commiss Sandral . Xkile

GTMARCHITECTS

(240)333-2001 FAX



Consultant

SITE PLAN

**ENGLE** RESIDENCE

3929 BALTIMORE ST., KENSINGTON, MD 20895

PERMIT SET 10/23/2020

Issue Description Date

19.0654 GTM Project No GTM Checked By LEO AS NOTED

Sheet Title

**COVER SHEET** 

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Ceramic Tile:

ASTM E 648.

ASTM E 662.

Architectural Woodwork and Trim:

not be accepted.

ASTM E 84, AND

Flat trim shall be clear pine or approved equal.

consistent with the best practices of the trade.

installed according to manufacturer's recommendations. When fastening two or more beams together, provide a minimum of two rows of 16 d nails 4. Replacement reserve: Contractor shall furnish to the Owner one unopened box of additional tiles for future repairs and maintenance work.

Provide ceramic tile and accessories in accordance with the Tile Council of American Specifications 137.1, in colors and patterns to be

Installation: comply with ANSI A108.1, ANSI A108.2, and the "Handbook for Ceramic Tile Installation" of the Tile Council of America.

Extend tile into recesses and under equipment and fixtures to form a complete covering without interruptions.

Terminate tile neatly at obstruction, edges, and corners, without disruption of pattern or joint alignment.

Replacement reserve: Contractor shall furnish Owner with one unopened box of additional tile for future repairs and maintenance.

All millwork trim and molding shall be installed accordingly to the quality of standards of the Architectural Woodwork Institute (AWI).

4. All corners of trim and siding are to be mitered, except inside corners of interior running trim which shall be coped. Exposed end grains will

5. All millwork and trim shall be installed by craftsmen with experience in work of this type. All work shall be first class in every regard and

Layout tile work and center the tile fields in both directions in each space or on each wall area.

Align joints when adjoining tiles on floor, base, trim, and walls are the same size.

All paint and primers to be Benjamin Moore or approved equal. Refer to schedule for colors and types.

Provide carpeting as indicated on the drawings. Refer to allowances on schedule sheet.

All surfaces to be painted shall receive one primer coat and two finish coats.

All paint shall be applied according to manufacturer's recommendations.

All interior trim and millwork shall conform to AWI "custom standards."

Installation of all vinyl composition tile (VCT) shall be done in a manner which conforms with:

Setting materials: comply with pertinent recommendations contained in the Tile Council of America "Handbook for Ceramic Tile Installation."

12. Provide 2x4 intermediate blocking at all bearing and non-bearing partitions.

Sill plates resting on concrete or masonry walls.

Exterior porch and deck framing, decking, and stairs.

All exterior wood trim shall be clear pine or redwood.

All trim shall be primed on both sides prior to installation

All outside corners shall be mitered. No butt joints will be accepted

lumber shall not be placed in contact with aluminum flashing or other aluminum components.

Sill plates resting on concrete slabs on grade.

Sleepers resting directly on concrete slabs.

Exterior wall sheathing shall be  $\frac{1}{2}$ " plywood unless noted otherwise.

exposed to weather during construction shall be Exposure I min.

clips at butt joints of roof sheathing.

Foundations: Adjacent soil including soil along the entire inside perimeter of foundation walls, along both sides of interior partition 14. MICRO-LAM L.V.L. (laminated veneer lumber) beams shall be manufactured by Trus Joist MacMillan or approved equal. Beams shall be

18. Exterior Wood Trim:

accordance with manufacturer's recommendations.

platform, porches, and equipment bases. Apply overall treatment only where attached concrete platform and porches are on fill or 16. The following wood elements are to be pressure treated with preservative:

13. All plywood shall be APA span rated. Use exterior grade plywood wherever edge of face will be exposed to weather. Interior plywood

15. TJI Floor Joists are to be manufactured by Trus Joist MacMillan or approved equal. Install per manufacturer's recommendations.

Joists which enter concrete or masonry walls and have less than  $\frac{1}{2}$ " clearance on tops, sides, and ends.

17. Fasteners, hangers, and metal accessories used in pressure treated wood construction shall be type 304 or 316 stainless steel. Treated

19. Exterior Synthetic Trim shall be "AZEK," with traditional smooth surface. Fasteners, joint cement, and installation procedures shall be in

Subflooring shall be  $\frac{3}{4}$ " tongue and groove plywood, glued and screwed to the floor joists as per APA recommendations.

Where spacing of roof structure members is 16" O.C., roof sheathing shall be  $\frac{1}{2}$ " plywood ( $\frac{3}{4}$ " where roofing is slate or tile). Where

spacing of roof structure members is 24" O.C., roof sheathing shall be  $\frac{5}{8}$ " plywood ( $\frac{3}{4}$ " where roofing is slate or tile). Provide "H"

TERMITE CONTROL SOIL TREATMENT

areas under slabs and footings.

CONCRETE

Treat soil with Bayer Corporation, Premise 75, in strict accordance with manufacturer's recommendations.

Along driplines of roof overhangs without gutters.

All concrete construction shall conform to the latest A.C.I. code 332.

• F/C = 3,000 PSI for foundation walls exposed to weather.

At plumbing penetrations through ground-supported slabs.

Other sites and locations as determined by licensed installer.

Where condensate lines from mechanical equipment drip or drain to soil.

Cement conforming to ASTM 150, and shall have a minimum 28-day compressive strength (F/C) as follows:

• F/C = 2,500 PSI for footings, interior slabs on grade (except garages) and fill in concrete blocks

slabs as an overall treatment. Treat soil materials before concrete footings and slabs are placed.

ground. Crawlspaces used as plenum spaces strictly follow manufacturer's recommendedations.

period of five (5) years from Substantial Completion, re-treat soil and repair or replace damage caused by termite infestation.

Special Warranty: Manufacturer's standard form, signed by Applicator and Contractor certifying that termite control work, consisting of applied

termiticide treatment, will prevent infestation of subterranean termites. If subterranean termite activity or damage is discovered during warranty

Concrete shall have natural sand fine aggregates and normal weight coarse aggregates conforming to ASTM C33, Type 1 Portland

Remove all extraneous sources of wood cellulose and other edible materials such as wood debris, tree stumps and roots, stakes, formwork,

bases; also along the entire outside perimeter, from grade to bottom of footing. Avoid soil washout around footings.

and construction waste wood from soil within and around foundations. Loosen, rake, and level soil to be treated except previously compacted

Slabs-on-Grade and Basement Slabs: Under ground-supported slab construction, including footings, building, slabs, and attached

walls, around plumbing pipes and electric conduit penetrating the slab, and around interior column footers, piers, and chimney

Crawlspaces: Soil under and adjacent to foundations as previously indicated. Treat adjacent areas including around entrance

Issue Description Date GTM Project N Checked By Drawn By

Sheet Title

**SPECIFICATIONS** 

Sheet No.

AS NOTED

COPYRIGHT, 2020 GTM ARCHITECTS, IN

LEGEND

WM

CLEAN OUT

UTILITY POLE

WATER VALVE

WATER WETER

OYER HEAD WIRE

LANDSCAPE AREA

WOODS LINE

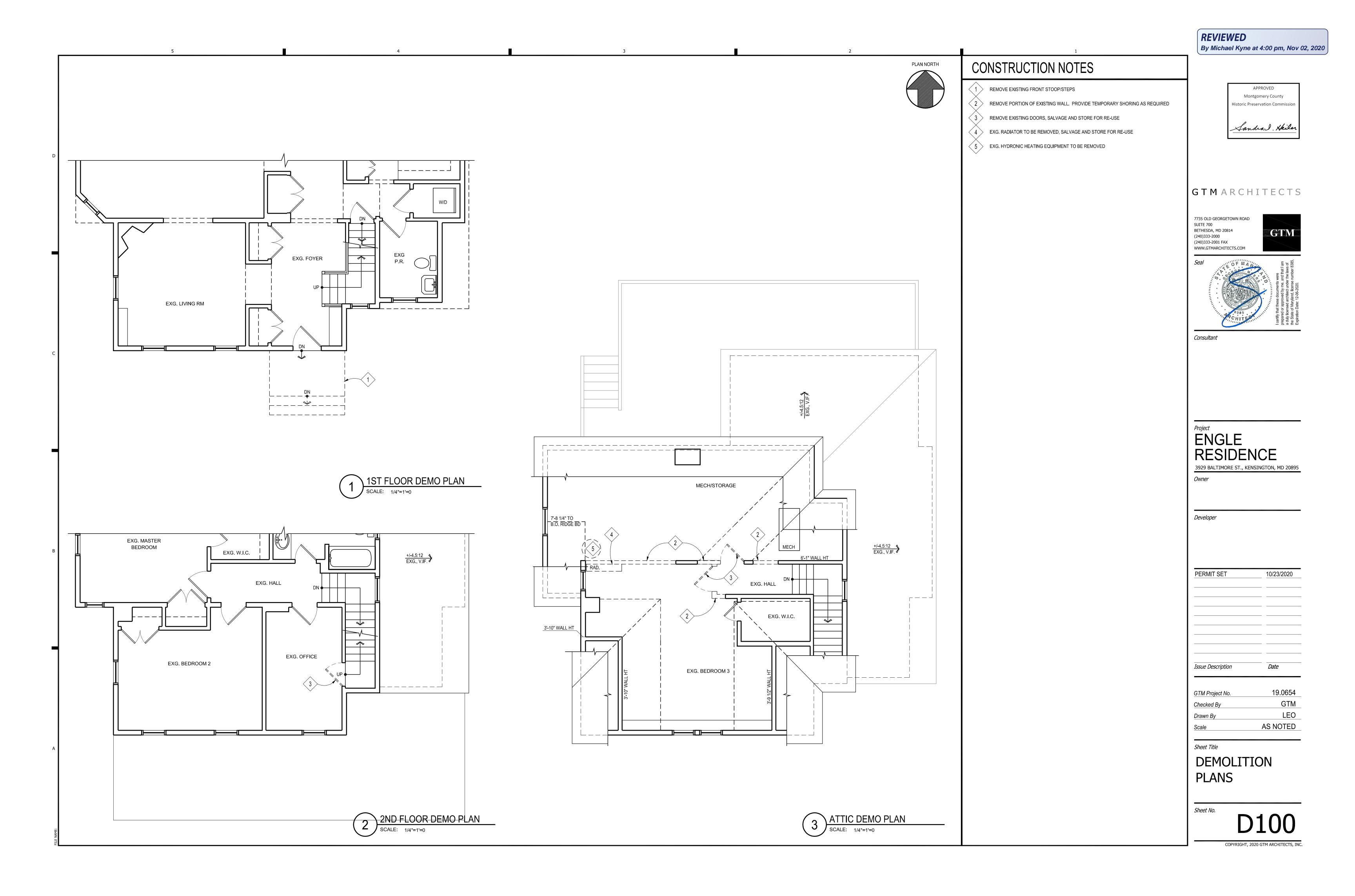
TRIPLE TREE WITH TYPE & SIZE

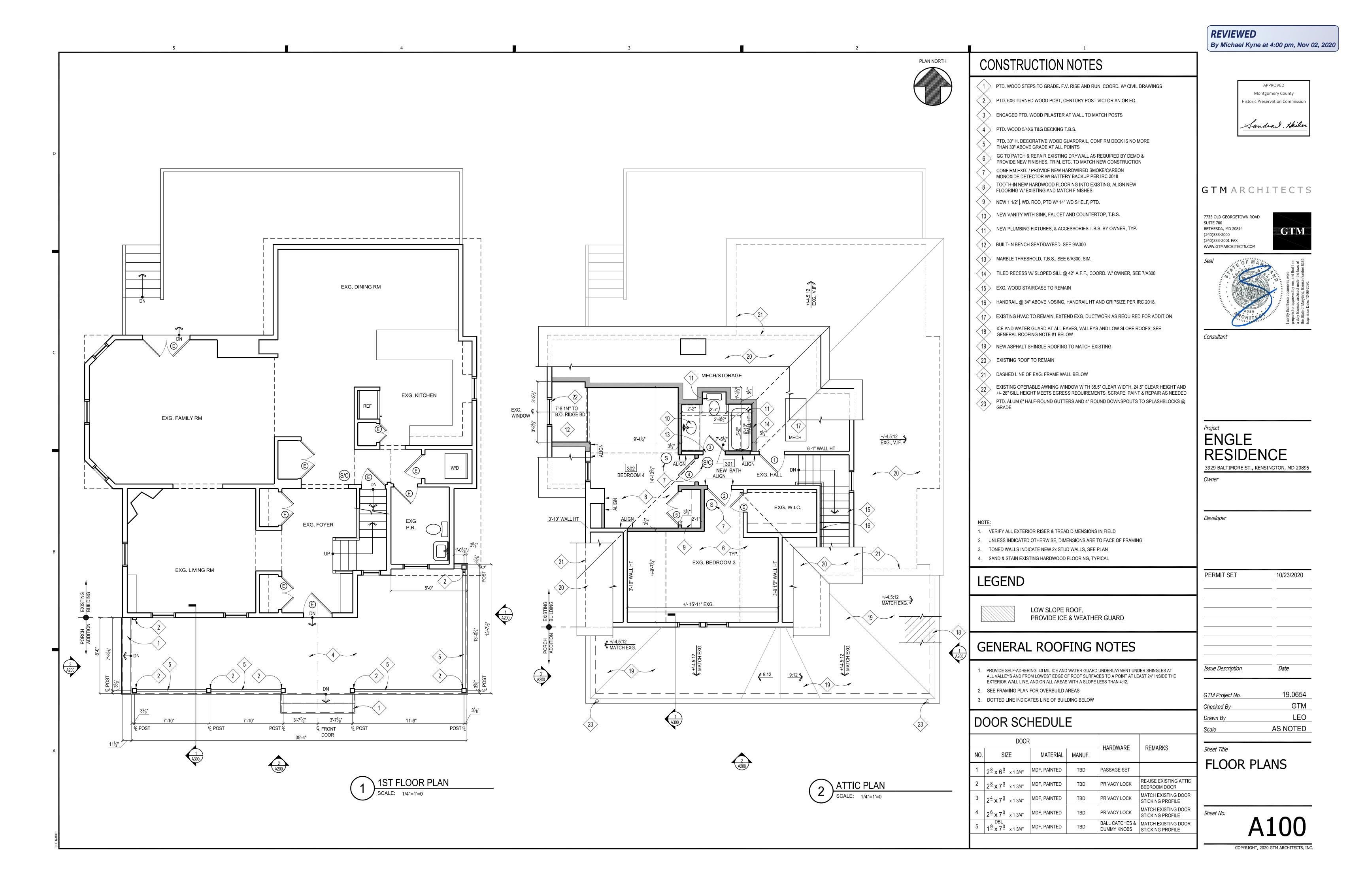
LIMIT OF DISTURBANCE

QUADRUPLE TREE WITH TYPE & SIZE

Sheet No.

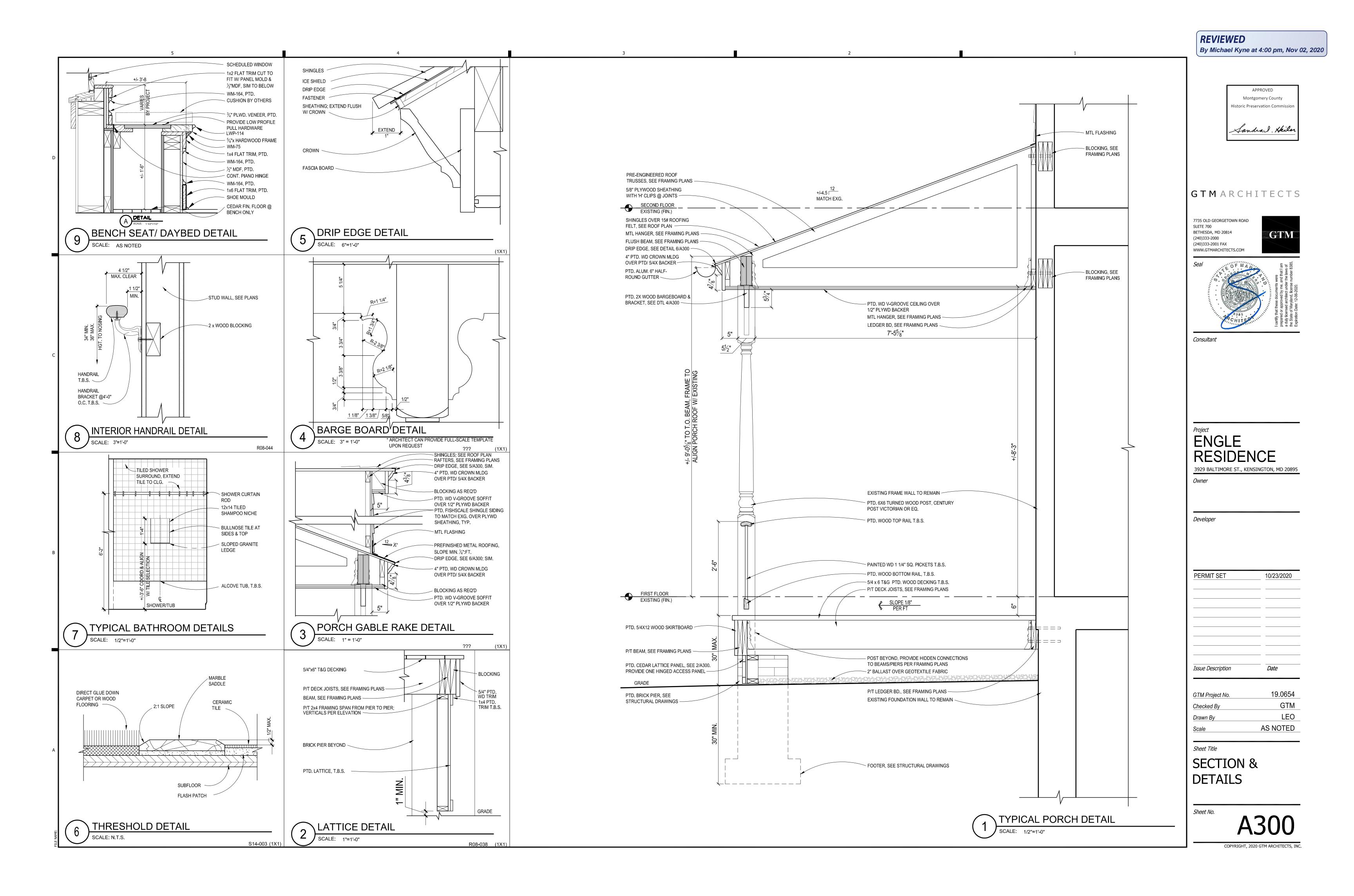
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REVIEWED

By Michael Kyne at 4:00 pm, Nov 02, 2020



### PRE-ENGINEERED WOOD TRUSSES

TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THESE SPECIFICATIONS AND WHERE ANY APPLICABLE DESIGN FEATURE IS NOT SPECIFIED HEREIN, DESIGN SHALL BE IN ACCORDANCE WITH APPLICABLE PROVISIONS OF LATEST EDITION OF NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (NDS) AMERICAN FOREST AND PAPER ASSOCIATION (AFPA), AND DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES (ANSI/TPI I), TRUSS PLATE INSTITUTE (TPI), AND CODES OF JURISDICTION. FABRICATE, SUPPLY AND ERECT WOOD TRUSSES AS SHOWN ON THE DRAWINGS AND AS SPECIFIED. WORK SHALL INCLUDE ALL ANCHORAGE, BLOCKING, CURBING, MISCELLANEOUS FRAMING AND BRACING.

LUMBER USED FOR TRUSS MEMBERS SHALL BE IDENTIFIED BY GRADE MARK OF A LUMBER INSPECTION AGENCY, AND SHALL BE AS SHOWN ON DESIGN DRAWINGS. TRUSSES SHALL BE HANDLED DURING FABRICATION, DELIVERY AND AT JOBSITE SO AS NOT TO BE SUBJECTED TO EXCESSIVE BENDING. TRUSSES SHALL BE UNLOADED ON SMOOTH GROUND TO AVOID LATERAL STRAIN. TRUSSES SHALL BE PROTECTED FROM DAMAGE THAT MIGHT RESULT FROM ON-SITE ACTIVITIES AND ENVIRONMENTAL CONDITIONS. PREVENT TOPPLING WHEN BANDING IS REMOVED.

HANDLE DURING INSTALLATION IN ACCORDANCE WITH HANDLING, INSTALLING AND BRACING WOOD TRUSSES (HIB-91), TPI, AND ANSI/TPI I-1995. INSTALLATION SHALL BE CONSISTENT WITH GOOD WORKMANSHIP AND GOOD BUILDING PRACTICES. TRUSSES SHALL BE SET AND SECURED LEVEL AND PLUMB, AND IN CORRECT LOCATION, TRUSSES SHALL BE HELD IN CORRECT ALIGNMENT UNTIL SPECIFIED PERMANENT BRACING IS INSTALLED. CUTTING AND ALTERING OF TRUSSES IS NOT PERMITTED. CONCENTRATED LOADS (FULL BUNDLES OF DECKING) SHALL NOT BE PLACED ATOP TRUSSES UNTIL ALL SPECIFIED BRACING HAS BEEN INSTALLED AND DECKING IS PERMANENTLY NAILED IN PLACE. ERECTION BRACING IS ALWAYS REQUIRED. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND FURNISHING THE MATERIALS USED FOR INSTALLATION AND PERMANENT BRACING.

STRUCTURAL ENGINEER OF RECORD SHALL APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO BUILDING OFFICIAL. BUILDING OFFICIAL SHALL APPROVE SHOP DRAWING PRIOR TO INSTALLATION. TRUSSES SHALL BE FABRICATED FROM APPROVED SHOP DRAWINGS.

MANUFACTURER SHALL SUBMIT 3 COPIES OF TRUSS DESIGN DRAWINGS BEARING SEAL OF PROFESSIONAL ENGINEER FOR APPROVAL PRIOR TO ERECTION AND ENGINEERING FRAMING PLANS FOR ALL FLAT CHORD TRUSSES. ALL TRUSS SHOP DRAWINGS MUST BE REVIEWED AND APPROVED IN WRITING, BY GENERAL CONTRACTOR, PRIOR TO SUBMITTAL OF SHOP DRAWINGS TO STRUCTURAL ENGINEER AND MUST INCLUDE THE FOLLOWING:

I. STAMP AND SIGNATURE OF ENGINEER, WHO IS REGISTERED IN THE STATE WHERE THE JOB IS TO BE CONSTRUCTED, RESPONSIBLE FOR PREPARATION OF ALL TRUSS DESIGN AND LAYOUT DRAWING. 2. ALLOWABLE LOADS IN LBS/EFFECTIVE NAIL OR PSI FOR LUMBER & PLATES USED AS ALLOWED BY ICBO, CURRENT ICBO REPORT NUMBER & BY SOUTHERN BUILDING CODE CONGRESS INTERNATIONAL. 3. STRESS REDUCTION FACTORS USED FOR PLATES.

4. TOP AND BOTTOM CHORD DESIGN LOADS IN PLF. 5. SIZE, GAUGE, AND EXACT LOCATION BY DIMENSION OF PLATES.

9.TRUSS CONNECTION HARDWARE REQUIREMENTS

MUST BE SHOWN ON TRUSS ENGINEERING SHEET.

6. LUMBER SPECIES AND GRADES USED. 7. NAME & TRADEMARK OF PLATE MANUFACTURER, TRUSS FABRICATOR & PROJECT NAME/LOCATION 8. CONCENTRATED LOAD REQUIREMENTS HAVE BEEN DESIGNED FOR AND SHOWN ON DOCUMENTS.

ALL TRUSSES MUST BE DESIGNED FOR UPLIFT LOADS. UPLIFT VALUES @ EACH TRUSS BEARING POINT

ALL ROOF TRUSSES SHALL BE ATTACHED TO PERPENDICULAR NON-LOAD BEARING WALLS WITH TRUSS CLIPS. CEILING GWB SHALL BE ATTACHED TO BLOCKING ON THE WALL AND NOT TO THE TRUSS FOR A DISTANCE OF 18" FROM THE WALL.

ALL FLOOR TRUSSES ON THE LOWEST FLOOR W/ TRUSSES SHALL BE ATTACHED TO PERPENDICULAR NON-LOAD BEARING WALLS WITH TRUSS CLIPS. CEILING GWB SHALL BE ATTACHED TO BLOCKING ON THE WALL AND NOT TO THE TRUSS FOR A DISTANCE OF 18" FROM THE WALL.

LIVE LOAD DEFLECTION SHALL NOT EXCEED 1 OR L/480 FOR FLOOR TRUSSES AND 1 OR L/360 FOR

THE MANUFACTURER SHALL SUPPLY ALL REQUIRED HANGERS, HOLD-DOWN CLIPS, AND OTHER SPECIAL

### MASONRY

ALL MASONRY WORK SHALL CONFORM TO THE APPLICABLE REQUIREMENTS OF BIA AND NCMA SPECIFICATION FOR CONCRETE MASONRY CONSTRUCTION (ACI 513.1-76) AND "SPECIFICATIONS FOR MASONRY STRUCTURE (ACI 530.I-02)" PUBLISHED BY THE AMERICAN CONCRETE INSTITUTE.

PROVIDE CONTINUOUS MASONRY BOND BEAM SPANNING ALL EXPANSION JOINTS # WALL INTERSECTIONS.

PROVIDE (2) #5 BENT BARS WITH 3-FOOT LEGS AT EVERY CORNER OR WALL INTERSECTION.

CONTINUOUS TIE OR BOND BEAMS SHALL BE REINFORCED WITH NOT LESS THAN 2 #5 CONTINUOUS BARS, LINTELS SHALL BE THE SIZES SHOWN AND REINFORCED AS INDICATED ON THE DRAWINGS.

REINFORCED MASONRY WALLS SHALL HAVE ALL REINFORCED CELLS FILLED WITH CONCRETE. CONCRETE MAY BE PLACED IN MAXIMUM VERTICAL LIFTS NOT TO EXCEED 4-FEET. ROUGHEN ALL SURFACES OF CONCRETE FILL WHICH ARE TO RECEIVE ADDITIONAL LIFTS ABOVE.

MASONRY WALLS SHALL HAVE "DUR-O-WALL" (OR APPROVED EQUAL) TRUSS TYPE HORIZONTAL REINFORCEMENT AT 16"00 VERTICALLY ABOVE GRADE AND 8"00 VERTICALLY BELOW GRADE. COORDINATE BRICK TIE BACK REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. UNLESS NOTED OTHERWISE, STOP ALL HORIZONTAL JOINT REINFORCING AT CONTROL JOINTS.

BRICK VENEER WALLS TO HAVE NON-CORROSIVE METAL TIES AT 16 oc VERTICALLY AND HORIZONTALLY AND COMPLY WITH ASTM A82 WITH AI53, CLASS B-2 COATING. MINIMUM WIRE DIAMETER SHALL BE 0.1875 INCHES. PROVIDE WEEP HOLES AT 24" AT BASE FLASHING.

PROVIDE MIN. 2 COURSES 8"x 16" SOLID BEARING AT BEAM & HEADER BEARING POINTS IN CMU

A36 STEEL LINTEL SIZES FOR OPENINGS PER 4" THICKNESS OF MASONRY WALL AS FOLLOWS: 4'-0" SPAN OR LESS L3"x 3" 1/2"x 5/16" 7'-6" SPAN OR LESS L5"x 3 1/2"x 5/16" 5'-6" SPAN OR LESS L4"x 3 1/2"x 5/16" 9'-0" SPAN OR LESS L6"x 3 1/2"x 5/16" PROVIDE MIN. 6" BEARING, EACH END & BRICK TIES, 16"oc @ 1st COURSE ABOVE LINTEL.

FILL SOLIDLY w/2,500psi ASTM C-476 GROUT, ALL BOND BEAMS, CELLS THAT ARE REINFORCED, WILL SECURE EXPANSION BOLTS, SILL PLATE ANCHOR BOLTS OR OTHER MECHANICAL ATTACHMENTS AND ALL CELLS BELOW GRADE.

REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM A-615, GRADE 60. SHOP FABRICATES REINFORCING BARS, WHICH ARE SHOWN TO BE HOOKED, OR BENT. PROVIDE A MINIMUM LAP OF 48 BAR DIAMETERS AT ALL SPLICES, UNLESS INDICATED OTHERWISE.

UNLESS OTHERWISE NOTED, ALL WALLS SHALL BE LAID IN RUNNING BOND. BOND CORNERS AND INTERSECTIONS OF LOAD-BEARING WALLS.

PROVIDE VERTICAL REINFORCING BARS OF THE GIVEN SIZE AND SPACING AS INDICATED. PROVIDE BARS AT ALL WALL CORNERS, INTERSECTIONS AND OPENINGS EDGES.

PROVIDE REBAR DOWELS FROM FOUNDATIONS TO MATCH VERTICAL REINFORCING SIZE AND SPACING. DOWELS SHALL HAVE STANDARD 90-DEGREE HOOKS AND LAP WITH THE FIRST LIFT OF REINFORCING.

PROVIDE BOND BEAM LINTELS AND BRICK SHELF ANGLES ABOVE ALL WALL OPENINGS.

PROVIDE JOIST & BEAM BEARING PLATES W/OTHER ACCESSORIES AS INDICATED, WITH 3 COURSES OF SOLIDLY GROUTED CMU BELOW ALL BEAM BEARINGS OVER A WIDTH OF 2'-8" CENTERED ON THE BEAM.

PROVIDE CMU CONTROL JOINTS AS INDICATED, W/ADDITIONAL JOINTS SUCH THAT THE SPACING BETWEEN JOINTS DOES NOT EXCEED A SPACING OF 3x WALL HEIGHT, 35' MAXIMUM. WHERE BEAMS OR LINTELS BEAR AT CMU CONTROL JOINTS, OFFSET \$ LAP THE VERTICAL REINFORCING AS

MASONRY CONTRACTOR SHALL PROVIDE ALL REQUIRED TEMPORARY BRACING DURING CONSTRUCTION.

### WOOD FRAMING

NAIL IN ACCORDANCE WITH RECOMMENDED WOOD FASTENING SCHEDULE IN APPLICABLE BUILDING CODES (LATEST EDITION/HIGH WIND REGION). PROVIDE BLOCKING, BRIDGING AND BRACING PER SAME CODE. AT A MIN., PROVIDE BRIDGING AT EACH END OF THE JOIST, AND ONE ROW OF SOLID BRIDGING BELOW ALL INTERIOR BEARING PARTITIONS.

FASTENERS: JOIST HANGERS, HURRICANE ANCHORS, POST BASES AND OTHER FRAMING ANCHORS ARE TO BE AS MANUFACTURED BY SIMPSON STONG-TIE, U.S.P., OR EQUAL, AND ARE TO BE USED IN STRICT ACCORDANCE WITH MANUFACTURER'S WRITTEN SPECIFICATIONS. ALL FASTENERS TO BE 16 GA. MIN. UNLESS NOTED OTHERWISE. PROVIDE GALV. FINISH UNLESS NOTED OTHERWISE. JOIST HANGERS SHALL BE MIN. 16 GA. WITH SIZE AND PROFILE TO SUIT APPLICATION (U.N.O.). PROVIDE JOIST HANGERS FOR ALL FLUSH FRAMED JOISTS. ALL FASTENERS IN CONTACT WITH

NUMBER OF MEMBERS IN THE BEAM BY ONE. THE CENTERLINE OF THE BEAM SHALL BE THE CENTERLINE OF THE SUPPORTING WALL STUDS. (UNLESS NOTED OTHERWISE ON PLAN) ALL MICRO-LAM BEAMS SHALL HAVE 3 STUDS (MIN. & EXCEED WIDTH OF BEAM). CONTINUE THESE STUDS TO THE FOUNDATION WITH INTERMEDIATE SUPPORTS THROUGH FLOOR, BETWEEN LOWER WALL TOP PLATE & UPPER WALL BOTTOM PLATE.

ALL EXTERIOR POSTS TO BE TREATED 6X6 (U.N.O.). NOTCH TOP OF POST FOR BEAM BRG. (3" MAX.) AND THRU BOLT BEAM TO POST WITH (2) 1/2" DIA. GALV. BOLTS. ALTERNATE: PROVIDE COLUMN CAP CONNECTION WITH #AC SERIES BY SIMPSON STONG-TIE OR EQ. PROVIDE SOLID BLOCKING BELOW ALL COLUMNS, TO TRANSFER LOAD DIRECTLY TO FRAMING/FOUNDATION BELOW

PROVIDE DOUBLE JOIST UNDER ALL PARTITIONS PARALLEL TO JOIST SPAN AND AROUND ALL FLOOR AND ROOF OPENINGS. SPACE & BLOCK IF PARTITIONS ABOVE IS A PLUMBING WALL. PROVIDE SOLID BLOCKING AT 12"00 BETWEEN JOISTS UNDER PARTITIONS ABOVE) WHICH ARE BLOCKS.

ALL MULTI-PLY BEAMS SHALL BE NAILED WITH 3 ROWS OF IOD NAILS AT 8"00 STAGGERED OR

BALLOON FRAME ALL END WALLS WITH CATHEDRAL CEILING (U.N.O.):

FASTEN GABLE-END WALL STUDS TO CEILING DIAPHRAGM BY FASTENING NAILER TO EACH STUD

WHERE DECKS FASTEN TO HOUSE FRAMING, PROVIDE CONTINUOUS TREATED LEDGER

ALL EXTERIOR WALLS SHALL BE STUDS AT 16" oc AS SPECIFIED ON PLANS WITH 7/16" OSB EXTERIOR SHEATHING. BLOCKING OF HORIZONTAL PANEL EDGES IS NOT REQUIRED. NAIL ALL

ROOF AND FLOOR FRAMING LAYOUTS ARE PROVIDED TO ILLUSTRATE CONDITIONS OF

CONSTRUCTION BRACING SHALL BE PROVIDED BY THE CONTRACTOR TO MAINTAIN THE BUILDING PLUMB AND TRUE. THIS BRACING SHALL REMAIN UNTIL THE SPECIFIED SHEARWALLS ARE

PRESCRIPTIVE BRACED WALL SEGMENTS SHALL HAVE STUDS AT 16" (MAX.) WITH 7" OSB EXTERIOR SHEATHING, BLOCKING OF HORIZONTAL PANEL EDGES IS NOT REQUIRED. NAIL ALL

SHEARWALLS SHALL HAVE STUDS @ 16"00 (MAX.) WITH 7 OSB EXTERIOR SHEATHING (U.N.O., SEE PLAN). BLOCKING OF HORIZONTAL PANEL EDGES IS REQUIRED. NAIL ALL SHEATHING PANEL EDGES WITH 8d NAILS AT 6 oc (U.N.O., SEE PLAN) AND INTERMEDIATE STUDS WITH 8d NAILS AT 12 oc

SHEAR WALL HOLD-DOWNS: ALL SHEAR WALLS SHOWN ON PLANS TO HAVE HOLD-DOWNS AT THE BASE AT EACH WALL END SHALL BE AS FOLLOWS:

\* AT UPPER FLOORS USE (2) SIMPSON HD8A'S OR (1) SIMPSON FTA7 AT EACH END OF SHEAR WALL SEGMENT AND EACH EXTERIOR CORNER OF BUILDING. (U.N.O., SEE PLAN) \* AT CONCRETE FOUNDATIONS USE (I) SIMPSON HD2A AT EACH END OF SHEAR WALL SEGMENT AND AT EACH EXTERIOR CORNER OF BUILDING. (U.N.O., SEE PLAN) \* AT PILE/GIRDER SUPPORTED FLOOR, USE (2) SIMPSON HD8A'S OR (1) SIMPSON FTA7 AT

\* PROVIDE 3 STUDS MIN. AT EACH HOLD-DOWN.(U.N.O., SEE PLAN)

ALL INTERIOR SHEAR WALLS SHOWN ON THE PLANS SHALL HAVE STRUCTURAL SHEATHING THAT EXTENDS TO THE UNDERSIDE OF THE FLOOR SHEATHING ABOVE. WHERE JOISTS RUN PARALLEL TO THE SHEAR WALL, PROVIDE A DBL.-JOIST ABOVE THE SHEAR WALL. WHERE JOISTS RUN

ALTERNATE POWER NAILS (FOR FRAMING MEMBERS ONLY, - 0.113 $\phi$  x 2%" FOR 8d NAILS \$ 0.131 $\phi$  x 3" FOR 16d NAILS PROVIDE DEFORMED SHANK NAILS AS REQD. BY U.L. RATINGS.

LATEST SPECIFICATION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.

PROVIDE WELDED CONNECTIONS TYPICALLY UNLESS OTHERWISE NOTED.

WELDS SHALL BE MADE ONLY BY WELDERS WHO HAVE BEEN PREQUALIFIED BY TESTS OF THE AMERICAN WELDING SOCIETY, PRESCRIBED IN THE STRUCTURAL WELDING CODE," AWS DI.I (LATEST EDITION).

ANY CONNECTION NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED AND DETAILED BY THE STRUCTURAL STEEL FABRICATOR. SEE THE TYPICAL BEAM CONNECTION DETAILS ON THE DRAWINGS.

MILL BOTTOM OF ALL COLUMNS AND FINISH TOP OF ALL BASE PLATES IN ACCORDANCE WITH A.I.S.C SPECIFICATIONS. BASE PLATES SHALL BE WELDED TO BOTTOM OF COLUMNS.

CONNECTIONS SHALL BE AISC STANDARD.

PROVIDE BASE PLATE FOR ALL STRUCTURAL STEEL BEAMS BEARING ON CONCRETE OR MASONRY. GROUT FOR SETTING BEARING SURFACES SHALL BE NON-SHRINK, NOT-STAINING, EQUAL TO "MASTERFLOW 713" BY THE MASTER BUILDERS CORPORATION.

SPECIFIED GROUT THICKNESS INCLUDES 1/4 INCH THICK LEVELING PLATES WHICH SHALL BE USED UNDER ALL BEAMS AND COLUMNS RESTING ON CONCRETE.

SCHEDULE OF CONSTRUCTION MATERIALS

THE COMPRESSIVE STRENGTH IS BASED 28-DAY COMPRESSIVE STRENGTH.

NORMAL WEIGHT: ASTM C90, GRADE N, Fm= 1500 PSI

ASTM C216, SEVERE WEATHER BRICK, TYPE FBX, Fm=2000 PSI

ASTM C270 PROJECTION SPECIFICATION MORTARS SHALL CONSIST OF

TYPE I PORTLAND CEMENT, TYPE S HYDRATED LIME AND APPROVED

AGGREGATE, WITH 1800 psi MINIMUM AVERAGE COMPRESSIVE STRENGTH

ASTM C270 PROJECTION SPECIFICATION MORTARS SHALL CONSIST OF

TYPE I PORTLAND CEMENT, TYPE M HYDRATED LIME AND APPROVED

ASTM C270 PROJECTION SPECIFICATION MORTARS SHALL CONSIST OF

TYPE I PORTLAND CEMENT, TYPE N HYDRATED LIME AND APPROVED

SPECIFICATION

SPECIFICATION

STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM

A500, GRADE B, UNLESS OTHER SIDE NOTED IN THE

PROJECT SPECIFICATIONS

STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM

A36 (36KSI), UNLESS OTHER SIDE NOTED IN THE

ALL OTHER STRUCTURAL STEEL, INCLUDING PLATES AND

<u> 11SCELLANEOUS SHAPES SHALL CONFORM TO ASTM A36 (36KSI).</u>

BOLTS FOR CONNECTING STRUCTURAL STEEL SHAPES SHALL BE ASTM A325-N, 3-INCH DIAMETER, UNLESS OTHERWISE

NOTED ON THE DRAWINGS OR IN THE PROJECT SPECIFICATION

450 | 135 |

300 | 125

825 | 175 |

550 | 165

3100 2150 285 750

725

575

2650 | 1650 | 285 |

STRUCTURAL GLUED LAMINATED TIMBER (GLULAM)

ENGINEERED WOOD PRODUCTS

PREFABRICATED PREFABRICATED WOOD I-JOISTS SHALL BE MANUFACTURED BY BOISE

DESIGN VALUES ARE FOR NORMAL LOAD DURATION AND DRY SERVICE

A COMPREHENSIVE DESCRIPTION OF DESIGN VALUE ADJUSTMENT FACTORS.

WOOD I-JOISTS CASCADE, LLC. OR APPROVED SUBSTITUTE. THE MANUFACTURER SHALL

SPECIFICATION

PLYWOOD/OSB DOC PSI, DOC PS2, CSA0437 OR CSA0325 ADVARYTECH,

CONDITIONS. SEE NDS OR MANUFACTURES SPECIFICATION FOR

STRUCTURE WOOD NOT ALLOWED.

P) FRAMING DESIGN VALUES ARE BASED ON SPF No.2.

FRAMING DESIGN VALUES ARE BASED ON SYP No.2.

325 | 125 | 425

425

425

565

375

o compzone Fb Tension zone Fv Fc⊥ Fc II Ex 106

Fbx Fby Fv Fc1 Fc11 Ex106

2400 | 1850 | 240 | 650 | 1600

1700 | 1800 | 180 | 560 | 1950

SUPPLY ALL REQUIRED HANGERS, WEB STIFFENERS, SQUASH BLOCKS,

MANUFACTURER SHALL SUBMIT ERECTION DRAWINGS TO THE ENGINEER

INSTALLED AND. BRACED IN ACCORDANCE WITH THE MANUFACTURER'S

PRIOR TO FABRICATION ALL PREFABRICATED WOOD I-JOISTS SHALL BE

BEVELED BEARING PLATES, AND OTHER SPECIAL HARDWARE. THE

1150

500

1650

525

PROJECT SPECIFICATIONS

STRUCTURAL STEEL I BEAMS SHALL CONFORM

TO ASTM A572 GRADE 50 (50 KSI).

ANCHOR BOLTS ANCHOR BOLTS SHALL CONFORM TO ASTM A307.

OR 4X

5X5 AND

ARGER(P)

2X4

2X6

5X5 AND

**LARGER** 

AND BEAMS

BEAMS

LARGER(B) 600

WELDING ELECTRODES SHALL BE E70 SERIES.

875

1500

1250

850

DIMENSION AND STRUCTURAL COMPOSITE LUMBER

HIGH STRENGTH NEW BILLET STEEL CONFORMING TO

ASTM A-615, GRADE 60 (60,000 PSI) - DEFORMED

AGGREGATE, WITH 750 psi MINIMUM AVERAGE COMPRESSIVE STRENGTH OI

AGGREGATE, WITH 2500 psi MINIMUM AVERAGE COMPRESSIVE STRENGTH

CONCRETE SUBJECTED TO FREEZE AND THAW CONDITIONS DURING

NORMAL WEIGHT: ASTM C145, GRADE N

OF 2-INCH CUBES AT 28-DAYS.

OF 2-INCH CUBES AT 28-DAYS.

2-INCH CUBES AT 28-DAYS.

LOCATION

FOOTINGS AND OTHER CONCRETE POURED AGAINST EARTH

FORMED CONCRETE NOT EXPOSED TO WEATHER OR EARTH

ASTM A-185

FORMED CONCRETE EXPOSED TO EARTH

REINFORCED MASONRY WALLS

SLABS ON GROUND, UNLESS OTHERWISE NOTED

BASEMENT WALLS & FDN NOT EXPOSED TO WEATHER

BASEMENT SLABS AND INTERIOR SLABS ON GRADE

BASEMENT WALLS, FDNS, EXTERIOR WALLS & OTHER

DRIVEWAYS, CURBS, WALKS, PATIOS, STEPS AND

UNHEATED GARAGE FLOORS EXPOSED TO WEATHER

CONSTRUCTION SHALL BE AIR-ENTRAINED (6% +/- 1%).

) CONCRETE SHALL BE AIR-ENTRAINED (6% +/- 1%).

CONCRETE EXPOSED TO WEATHER

STONE VENEER OWNER APPROVED

CONCRETE BRICK ASTM C95 TYPE I, GRADE 8

HOLLOW CMU

FACE BRICK

SOLID CMU

ABOVE GRADE

SINGLE WYTHE

BELOW GRADE

MATERIAL

WELDED WIRE

FABRIC

SHAPES

MATERIAL

UNTREATED

FRAMING

FRAMING

LVL (2.0E)

MATERIAL

UNTREATED

FRAMING

CONNECTION

MASONRY

REINFORCING

STEEL

STRUCTURAL

MOOD

COMP. STRENGTH SLUMP

4" +/- 1"

4" +/- 1

3000 PSI (2)

3000 PSI (3)

3500 PSI (3)

3000 PSI

PRESSURE TREATED WOOD SHALL BE Z-MAX OR TRIPLE ZINC COATED, U.N.O.

THE NUMBER OF WALL STUDS AT BEARING POINTS OF 2X MEMBER BEAMS SHALL EXCEED THE

PARALLEL TO THE JOISTS BUT NOT DIRECTLY OVER THE JOISTS. BLOCKING SHALL BE NOT LESS THAN 2" IN. THICKNESS & SHALL MATCH THE DEPTH OF THE JOISTS. TRUSSES MAY USE TRUSS

BOLTED WITH 1/2" DIA. BOLTS AT 16"oc STAGGERED (U.N.O.).

PROVIDE COLLAR TIES OF IX6 BOARDS AT UPPER I/3 DOWN FROM RIDGE BEAMS SPACED 48"0c MAXIMUM. (FOR CONVENTIONAL FRAMING)

2x4 @ 16"oc UP TO 9'-0", 2x6 @ 16"oc UP TO 14'-0" \$ 2x8 @ 16"oc UP TO 18'-0"

AND BY FASTENING CEILING TO NAILER WITH 8d NAILS AT 6"oc

THRU-BOLTED TO FLOOR STRUCTURE WITH (2) 1/2" DIA. BOLTS AT 16"oc PROVIDE HOT-DIPPED GALV. JST. HANGER TO LEDGER.

REQUIRED PANEL EDGES WITH 8d NAILS AT 6 oc AND INTERMEDIATE STUDS WITH 8d NAILS AT

CONSTRUCTION AND DO NOT NECESSARILY INDICATE SPECIFIC QUANTITIES OF MATERIALS OR COMPONENTS REQUIRED FOR CONSTRUCTION.

TOTALLY INSTALLED.

SHEATHING PANEL EDGES WITH 8d NAILS AT 6 OC AND INTERMEDIATE STUDS WITH 8d NAILS AT

EACH END OF SHEAR WALL SEGMENT AND AT EACH EXTERIOR CORNER OF BUILDING. (U.N.O., SEE

\* PROVIDE TRIPLE JOISTS BELOW SHEAR WALLS THAT RUN PARALLEL TO FLOOR FRAMING.(U.N.O., SEE PLAN)

PERPENDICULAR, PROVIDE 2X BRIDGING ABOVE SHEAR WALL AND "TOOTH" PLYWOOD AROUND JOISTS. NAIL THROUGH FLOOR SHEATHING ABOVE INTO WALL WITH (2) ION NAILS AT 4" oc

FABRICATION AND ERECTION OF ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE



50 PSF

42 PSF

42 PSF

32 PSF

15 PSF

42 PSF

52 PSF

50 PSF

70 PSF

60 PSF

100 PSF

THE FOLLOWING CODES AND STANDARDS, INCLUDING ALL SPECIFICATIONS REFERENCED WITHIN, SHALL

INTERNATIONAL RESIDENTIAL CODE (IRC), INTERNATIONAL CODE COUNCIL, INC., 2015

THE PROJECT. USE THE LATEST EDITIONS UNLESS NOTED OTHERWISE.

ACI MANUAL OF CONCRETE PRACTICE - PARTS I THROUGH 5 - 2011

WITH EXCEPTION, IF ANY, AS INDICATED IN THE SPECIFICATIONS).

30 PSF

30 PSF

30 PSF

20 PSF

10 PSF

30 PSF

40 PSF

50 PSF

40 PSF

60 PSF

BASIC WIND SPEED (3 SEC GUST)

WIND PRESSURE (ROOF AVG.)

WIND PRESSURE (WALL AVG.)

SNOW EXPOSURE FACTOR (C.)

SNOW LOAD IMPORTANCE FACTOR

WIND LOAD IMPORTANCE

GROUND SNOW LOAD (Pa)

THERMAL FACTOR

SNOW LOAD (ROOF)

RAIN-ON-SNOW

PEAK ACCELERATION (A)

SOIL PROFILE TYPE

FOUNDATION MUST BE RE-EVALUATED.\*

SOIL BEARING CAPACITY

TO INSTALLATION.

SEISMIC DESIGN CATEGORY

MINIMUM SNOW LOAD

SEISMIC PEAK VELOCITY-RELATED ACCELERATION(A)

SEISMIC HAZARD EXPOSURE GROUP

BASIC STRUCTURAL SYSTEM AND

RESPONSE MODIFICATION FACTOR (R

DEFLECTION AMPLIFICATION FACTOR (C)

REVIEWED

BACK FILL 60 PCF EQUIVALENT FLUID WEIGHT, UNLESS OTHERWISE NOTED

ANALYSIS PROCEDURE (ELEP OR MAP)

SEISMIC RESISTING SYSTEM

WIND EXPOSURE CATEGORY

AMERICAN INSTITUTE OF STEEL CONSTRUCTION.

AND STEEL INSTITUTE, AISI S100-2007.

ASSOCIATION.

ROOF TRUSSES

ATTIC FLOORS (TYP)

NO STORAGE

SLEEPING ROOMS

OTHER FLOORS

GARAGE FLOORS

LTD STORAGE

ROOF LIVE LOAD DESIGN

RAFTERS

DECKS

BALCONY

STAIRS

WIND LOAD

SNOW LOAD

CLEAR COVER (IN)

OF SLAB

MID-DEPTH

OF WALL

MANUAL OF STANDARD PRACTICE, CONCRETE REINFORCING STEEL INSTITUTE.

STRUCTURAL WELDING CODE ANSI/AWS D 1.1-92, AMERICAN WELDING SOCIETY.

DESIGN MANUAL FOR FLOOR DECKS AND ROOF DECKS, STEEL DECK INSTITUTE.

SOCIETY OF CIVIL ENGINEERS.

APPLY TO THE DESIGN, CONSTRUCTION, QUALITY CONTROL AND SAFETY OF ALL WORK PERFORMED ON

MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES (ANSI/ASCE 07-10), AMERICAN

MANUAL OF STEEL CONSTRUCTION - 13TH EDITION, AMERICAN INSTITUTE OF STEEL CONSTRUCTION

STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, AND AISC CODE OF STANDARD PRACTICE

MANUAL OF STEEL CONSTRUCTION, VOLUME II CONNECTIONS, ASD 13TH EDITION/LRFD IST EDITION,

SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS AMERICAN IRON

BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-10/ASCE 5-05/TMS 402-05)

NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION/ 2012, AMERICAN FOREST & PAPER

DESIGN LOADS

DEAD LOADS

12 PSF

12 PSF

12 PSF

5 PSF

12 PSF

12 PSF

50 PSF

10 PSF

10 PSF

20 PSF

30 PSF

16 PSF

\*ADDITIONAL DRIFT AND SLIDING SNOW LOADS HAVE BEEN CONSIDERED WHERE APPLICABLE.\*

\*IF IT IS DETERMINED THAT THE SOIL CAPACITY AT THE SITE IS LESS THAN 2000 PSF, THE

MECHANICAL UNITS & OTHER EQUIPMENT SUPPORTED BY THE STRUCTURE WITH WEIGHTS IN

EXCESS OF 200# SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR

By Michael Kyne at 4:01 pm, Nov 02, 2020

APPROVED

Montgomery County

storic Preservation Commissi

Sandral. Keiler

10 PSF (TOP & BOTTOM)

10.0 PSF

27.9 PSF

30 PSF

1.00

0.07

ELEP

2000 PSF

18 PSF

20 PSF

23.0 PSF

2 (UNKNOWN)

LOAD BEARING WALL SYSTEM LIGHT

FRAMED WALLS W/SHEAR PANELS

EXPOSURE B

BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318-11, AMERICAN CONCRETE

( INCLUDING SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, SPECIFICATION FOR

DETAILING FOR STEEL CONSTRUCTION, AMERICAN INSTITUTE OF STEEL CONSTRUCTION.

SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-05/ASCE 6-05/TMS 602-05).

341 W. PATRICK STREET FREDERICK, MD 21701 240-815-6760 301-748-2769

### 3929 BALTIMORE ST. KENSINGTON MD 20895

GTM ARCHITECTS

ARCHITECT

CONTRACTOR:

CONTRACTOR

PROFESSIONAL CERTIFICATION. I HEREBY CERTIFY THAT

THESE DOCUMENTS WERE PREPARED OR APPROVED BY

ME, AND THAT I AM A DULY LICENSED PROFESSIONAL

ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND.

LICENSE NO.: 24518

EXPIRATION DATE: 09-21-2021



SCALE: AS NOTED		
DRAWN BY: 5L	CHECKED BY:	JMU

DATE:

ISSUE:

IS	SSUED FOR PERMITS	04-27-2020
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RI	EVISION:	
1	FRAMING REVISION	10-29-2020
2		
3		
2 3 4		
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7		
8		
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GENERAL STRUCTURAL NOTES & SCHEDULES

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### CONCRETE

ALL CONCRETE SHALL BE MADE IN ACCORDANCE WITH DESIGN MIXES WHICH ARE TO BE APPROVED BY THE ARCHITECT OR ENGINEER PRIOR TO CASTING ANY CONCRETE. MIXES SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTION ACI 318. ALL PLAIN CONCRETE TO CONFORM TO ACI 318.1 AND ACI 332 GUIDE TO RESIDENTIAL CAST IN PLACE CONCRETE CONSTRUCTION. MIXES SHALL HAVE A MINIMUM CEMENT CONTENT OF 520 LB. PER CUBIC YD. MAXIMUM WATER/CEMENT RATIO OF 0.53 FOR INTERIOR CONCRETE PROTECTED FROM FREEZING AND 0.45 FOR ALL EXTERIOR EXPOSED CONCRETE.

CONCRETE MATERIALS SHALL CONFORM TO ASTM CI50, TYPE I FOR PORTLAND CEMENT AND ASTM C33 FOR AGGREGATES. WATER-REDUCING ADMIXTURES SHALL CONFORM TO ASTM C494, TYPE A (FREE OF CALCIUM CHLORIDES), AIR-ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C260, AND HIGH-RANGE WATER REDUCERS (SUPER-PLASTICIZERS) SHALL CONFORM TO ASTM C494, TYPE F. FLY ASH SHALL COMPLY WITH ASTM C619 FOR CLASS F AND SHALL NOT BE PROPORTIONED IN MIXES WITH MORE THAN 20% CEMENT BY WEIGHT. LIQUID-MEMBRANE CURING COMPOUNDS SHALL BE HIGH-SOLIDS, WATER AND ACRYLIC-BASED, COMPLYING WITH ASTM C309 AS TESTED UNDER ASTM CI56. SLUMP OF THE CONCRETE SHALL BE A MINIMUM OF 4-INCHES AND A MAXIMUM OF 6-INCHES. SEE THE PROJECT SPECIFICATIONS. THE COMPRESSIVE STRENGTH IS BASED 28-DAY COMPRESSIVE STRENGTH.

SLAB ISOLATION JOINTS: PROVIDE PRE-MOLDED JOINT FILLER AROUND ALL PIPING, PIERS \$

ALL CONCRETE TO BE PLACED IN THE CELLS OF CONCRETE MASONRY UNITS (CMU BLOCK FILL), OR IN THE VOIDS OF BRICK MASONRY CONSTRUCTION, SHALL CONTAIN PEA GRAVEL (3/8" STONE) IN LIEU OF COARSE AGGREGATE. THE CONCRETE MIX SHALL CONTAIN A HIGH-RANGE WATER REDUCER (SUPERPLASTICIZER). SLUMP OF THE CONCRETE SHALL BE A MINIMUM OF 6" AND A MAXIMUM OF 9". SEE THE PROJECT SPECIFICATIONS.

ALL EXTERIOR CONCRETE AND CONCRETE EXPOSED TO WEATHER SHALL BE AIR-ENTRAINED, 6% +/-1%. USE OF ADDITIVES SHALL NOT BE PERMITTED UNLESS SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER. USE OF ADDITIVES CONTAINING CALCIUM CHLORIDE SHALL NOT BE PERMITTED. DO NOT USE HIGH-RANGE WATER REDUCING ADMIXTURES IN AIR-ENTRAINED CONCRETE. CONFORM TO ASTM C260.

ADDITION OF WATER TO THE CONCRETE AT THE JOB SITE FOR THE PURPOSE OF INCREASING THE SLUMP OR FOR RETEMPERING THE CONCRETE WHICH HAS BEGUN TO SET IS STRICTLY PROHIBITED. SEE THE PROJECT SPECIFICATIONS FOR REQUIREMENTS OF WATER ADDITION TO CONCRETE AT THE JOBSITE.

SLABS ON GRADE SHALL BE 4" THICK CONCRETE AND REINFORCED w/6x6 WI.4xWI.4 WWF. WELDED WIRE FABRIC SHALL BE SUPPORTED ON HIGH CHAIRS SO THAT THE FABRIC IS POSITIONED AT MID-DEPTH OF THE SLAB THICKNESS. LAP ONE FULL MESH PLUS 2" AT SPLICES IN EACH DIRECTION. PLACE CONCRETE OVER 6 MIL. POLYETHYLENE VAPOR BARRIER AND 4" MINIMUM COURSE AGGREGATE OR AS RECOMMENDED BY SOILS ENGINEER. THE AGGREGATE LAYER SHALL BE PLACED OVER FIRM NATURAL SUBGRADE OR ON COMPACTED AND CONTROLLED FILL. FILL UNDER SLABS SHALL BE COMPACTED IN 8 INCH LAYERS TO 95% MAX. DENSITY. USE AIR-ENTRAINED AT ALL EXTERIOR SLABS.

CONCRETE FOR SLABS-ON-GRADE SHALL BE PLACED IN A SEQUENCE AND MANNER THAT IS CONSISTENT WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE. LOCATE CONSTRUCTION AND CONTROL JOINTS IN SUCH A WAY TO MINIMIZE THE EFFECTS OF SHRINKAGE OF THE CONCRETE SLAB SECTIONS. SUBMIT TO THE ARCHITECT/ENGINEER THE SEQUENCE AND METHOD OF CASTING CONCRETE SLABS-ON-GRADE PRIOR TO PLACING THESE ELEMENTS. POUR SLABS IN ALTERNATE PANELS WITH A MAXIMUM OF 600 SF AND PROVIDE CONTROL AND CONSTRUCTION JOINTS AT 15'-O" MAXIMUM OR AS REQUIRED TO PREVENT UNCONTROLLED CRACKING.

SLAB CONTROL JOINTS: SAW CUT OR FORM TO 1/3 SLAB DEPTH. SPACE NO MORE THAN 15 FEET APART. DISCONTINUE WELDED WIRE FABRIC AT CONTROL JOINTS. PROVIDE JOINTS ON GROUND SUPPORTED SLABS IN RECTANGULAR CONFIGURATION, WITH THE LONGER SIDE NO MORE THAN ONE-AND-ONE-HALF TIMES THE LENGTH OF THE SHORTER SIDE.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR FURNISHING AND INSTALLING ANCHOR BOLTS, CLIPS, INSERTS, CONNECTION PLATES, SLEEVES, SLOTS AND OTHER REQUIRED ITEMS IN ACCORDANCE WITH THE CONTRACT DRAWINGS, AND IN COOPERATION WITH OTHER TRADES PRIOR

ALL REINFORCING SHALL BE DETAILED, FABRICATED, AND PLACED IN ACCORDANCE WITH ACI'S MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES, (ACI-315). DETAILS OF REINFORCEMENT SHALL CONFORM TO ACI 318, ACI 315, AND CRSI STANDARDS.

ALL REINFORCING STEEL (INCLUDING WELDED WIRE FABRIC) SHALL BE SECURELY TIED AND ANCHORED IN PLACE TO PREVENT DISLOCATION DURING THE PLACING OPERATION.

OTHER MATERIAL WHICH MAY INHIBIT THE BOND BETWEEN THE STEEL AND CONCRETE.

REINFORCING STEEL SHALL BE CLEAN OF MUD, DEBRIS, LOOSE RUST, CEMENT, GROUT, OR ANY

PROVIDE 8' X 8' CORNER BARS TO MATCH ALL HORIZONTAL REINFORCING IN WALLS AND FOOTINGS. ALL LAPS SHALL BE A MINIMUM OF 36 BAR DIAMETERS. PROVIDE DOWELS BETWEEN ALL FOOTINGS, WALLS AND PIERS TO MATCH SIZE AND SPACING OF VERTICAL REINFORCING.

DRY PACK SHALL CONSIST OF SIKA GROUT 212 OR APPROVED SUBSTITUTE. INSTALL PER MANUFACTURERS RECOMMENDATIONS.

FLITCH BEAMS SHALL BE SIZED AS INDICATED ON THE DRAWINGS, USING #2 SPF MINIMUM AND A-86 STEEL PLATE. USE TWO ROWS OF 1/2" DIAMETER THROUGH BOLTS 2" FROM TOP AND BOTTOM, SPACED 16" oc AT TOP AND 32" oc AT THE BOTTOM. BEGIN BOLTING ROWS 6" FROM ENDS. STEEL FLITCH PLATES MUST BE EITHER FULL LENGTH OR FULL MOMENT BUTT SPLICE.

WOOD EXPOSED TO THE ELEMENTS, WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND WOOD DESIGNATED "TREATED" SHALL BE #2 GRADE SOUTHERN PINE OR BETTER # PRESSURE IMPREGNATED WITH ALKALINE COPPER QUATERNARY (ACQ) IN ACCORDANCE WITH AMERICAN WOOD PRESERVERS ASSOCIATION (AWPA) STANDARD C2, WITH A MIN. RETENTION OF 0.40 LBS. PER CUBIC FOOT OF WOOD, MIN. DEPTH OF PENETRATION SHALL BE 2.5" OR 85% OF THE SAPWOOD.

ALL STUDS SHALL BE INSTALLED IN ACCORDANCE WITH WFOPA. MEMBERS ARE NOT TO BE DRILLED IN EXCESS OF NDS OR LOCAL CODE REQUIREMENTS, WHICHEVER IS MORE STRINGENT ALL POSTS AND MULTIPLE STUDS SHALL BE RUN CONTINUOUSLY TO SOLID BEARING ON FOUNDATION WALL OR BEAMS, PROVIDE SOLID BLOCKING AT FLOORS. COLUMNS SHALL BE ADEQUATELY ANCHORED TO PREVENT INTERNAL DISPLACEMENT.

FRAME CHIMNEYS: FRAME CHIMNEYS SHALL BE CONSTRUCTED OF MINIMUM #2 SPF STUDS MAXIMUM 16" oc USE 2 X 4 IF CHIMNEY EXTENDS LESS THAN 8' ABOVE ROOF, OTHERWISE USE 2 X 6. SHEATH WITH 1/2" APA OR APPROVED SUBSTITUTE RATED SHEATHING CONTINUOUS ACROSS PLATES AND JOISTS, GLUE, AND NAIL WITH 8D NAILS @ 6 oc SECURE TO ROOF. STUDS MUST BE CONTINUOUS ACROSS ROOF INTERSECTION.

NO STRUCTURAL MEMBER SHALL BE OMITTED, NOTCHED, CUT, BLOCKED OUT OR RELOCATED WITHOUT PRIOR APPROVAL BY THE DESIGNER OR STRUCTURAL ENGINEER. DO NOT ALTER SIZES OF MEMBERS NOTED WITHOUT APPROVAL OF BOTH.

CUTTING OF WOOD BEAMS, JOISTS AND RAFTERS SHALL BE LIMITED TO CUTS AND BORED HOLES NOT DEEPER THAN ONE-SIXTH THE MEMBER DEPTH AND SHALL NOT BE LOCATED WITHIN THE MIDDLE THIRD OF THE SPAN. NOTCHES LOCATED CLOSER TO SUPPORTS THAN THREE TIMES THE MEMBER DEPTH SHALL NOT EXCEED ONE-FIFTH THE DEPTH. HOLES BORED OR CUT INTO JOISTS SHALL BE MIN. 2" CLEAR FROM THE TOP OR BOTTOM OF THE JOIST AND THE HOLE DIAMETER SHALL NOT EXCEED ONE-THIRD OF THE JOIST DEPTH.

THERE SHALL NOT BE LESS THAN ONE LINE OF BRIDGING IN EVERY EIGHT FEET OF SPAN IN FLOOR, ATTIC AND ROOF FRAMING. THE BRIDGING SHALL CONSIST OF NOT LESS THAN ONE BY THREE INCH LUMBER DOUBLE NAILED AT EACH END OR OF EQUIVALENT METAL BRACING OF EQUAL RIGIDITY. MIDSPAN BRIDGING IS NOT REQUIRED FOR FLOOR, ATTIC OR ROOF FRAMING WHERE JOIST DEPTH DOES NOT EXCEED TWELVE INCHES NOMINAL. BLOCK ALL STUD WALLS AT MAXIMUM INTERVALS OF EIGHT FEET WITH A MINIMUM OF TWO-BY SOLID MATERIAL WITH TIGHT JOINTS. PROVIDE TWO-BY FIRE STOPS AT MID-POINT OF STUD WALLS.

UNLESS NOTED OTHERWISE, BRACE EXTERIOR CORNERS OF BUILDING WITH I X 4 DIAGONALS, LET INTO STUDS, OR 4 X 8 PLYWOOD SHEET OF THICKNESS TO MATCH THAT OF SHEATHING, OR WITH METAL STRAPS. LAP PLATES AT ALL CORNERS.

APPROVED Montgomery County istoric Preservation Commissio Sandral. Keile

MAXIMUM

HEIGHT

(Hunbal)

4,5,5'-4"

4,5

6'-4"

4,5

6

4,4'-4"

8" CONCRETE WALL (D) VERTICAL REINFORCEMENT

SW \$ SP

(30pcf)

N/A

N/A

N/A

N/A

N/A

N/A

N/A

N/A

SOIL CLASSIFICATIONS

(EQUIVALENT FLUID PRESSURE)

(45 pcf)

N/A

N/A

N/A

N/A

N/A

N/A

N/A

AND ML INORGANIC CL

#4 @ 24"oc | #4 @ 18"oc

N/A | #4 @ 24"oc

N/A #4 @ 24"oc

N/A #4 @ 20"oc

AND ML INORGANIC CL

(60 pcf)

N/A

N/A

N/A

N/A

#4 @ 20"oc

N/A

#4 @ 20"oc

N/A

(45 pcf)

N/A

N/A

N/A

N/A

N/A

N/A

N/A

(45 pcf)

N/A

N/A

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N/A

#4 @ 20"oc | #4 @ 16"oc

#4 @ 18"oc | #4 @ 12"oc

N/A | #4 @ 18"oc

AND ML INORGANIC C

N/A | #4 @ 16"oc

N/A #4 @ 18"oc

(60 pcf)

N/A

N/A

N/A

N/A

N/A

#4 @ 48"oc

N/A

 $e:\label{localization} e:\label{localization} e:\label{localizatio$ 

#4 @ 18"oc | #4 @ 16"oc

N/A

#4 @ 18"oc | #4 @ 12"oc

N/A

#4 @ 18"oc | #4 @ 16"oc

#4 @ 24"oc | #4 @ 18"oc

N/A

N/A

N/A

#4 @ 36"oc

FOUNDATION WALL REINFORCEMENT SCHEDULE

MAXIMUM

WALL

HEIGHT (H)

VERTICAL-

REBAR

HORIZL

REBAR

(SEE NOTE

By Michael Kyne at 4:01 pm, Nov 02, 2020

**REVIEWED** 

## STRUCTURAL ENGINEERING UNLIMITED, LLC 341 W. PATRICK STREET

FREDERICK, MD 21701 240-815-6760 301-748-2769

### MISCELLANEOUS

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL SAFETY REGULATIONS, PROGRAMS AND PRECAUTIONS RELATED TO ALL WORK ON THIS PROJECT AND FOR THE PROTECTION OF PERSONS AND PROPERTY EITHER ON OR ADJACENT TO THE PROJECT AND SHALL PROTECT SAME AGAINST INJURY, DAMAGE OR LOSS.

THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE STRUCTURE. SUCH LOADS SHALL NOT EXCEED THE CAPACITY OF THE STRUCTURE AT ANY TIME. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION, AND ANY TEMPORARY BRACING OR SUPPORT REQUIRED TO ACCOMMODATE THE CONTRACTOR'S MEANS AND METHODS ARE

THE CONTRACTOR IS TO VERIFY ALL OPENING SIZES AND LOCATIONS WITH THE REQUIREMENTS OF OTHER TRADES PRIOR TO FABRICATION AND ERECTION.

THE RESPONSIBILITY OF THE CONTRACTOR.

TEMPORARY SHORING AND BRACING DURING CONSTRUCTION

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS, AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR SEEING THAT THE WORK OF ALL TRADES IS COORDINATED WITH STRUCTURAL WORK.

EARTH RETAINING WALLS, OTHER THAN CANTILEVERED TYPE WALLS, SHALL BE ADEQUATELY BRACED UNTIL CONCRETE FOR SUPPORTING SLABS HAS BEEN PLACED AND ALL CONCRETE HAS CURED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DESIGNING, FURNISHING, ERECTING AND REMOVING ANY

THE ARCHITECT AND ENGINEER SHALL BE NOTIFIED AT THE PROPER TIME WHEN ALL ITEMS ARE READY FOR OBSERVATION. SUFFICIENT NOTICE SHALL BE GIVEN BY THE CONTRACTOR TO ALLOW FOR SCHEDULING OF OBSERVATIONS.

SAFETY REGULATIONS SHALL BE STRICTLY FOLLOWED BY THE CONTRACTOR OR SUBCONTRACTOR DURING ALL TIMES OF WORK ON THIS PROJECT. THE ARCHITECT OR ENGINEER SHALL NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR ACTS OF OMISSIONS OF THE CONTRACTOR, SUBCONTRACTORS, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

ALL SPECIALTY BOLTS, INCLUDING EXPANSION TYPE AND EPOXY TYPE ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTRUCTIONS.

THE CONTRACTOR SHALL PROTECT FROM DAMAGES EXISTING BUILDING(S), OWNER EQUIPMENT, ROADS, WALKS AND UTILITIES. THE CONTRACTOR SHALL MAINTAIN THESE DURING THE COURSE OF THE WORK, AND SHALL REPAIR ALL DAMAGES AT NO ADDITIONAL EXPENSE TO THE OWNER.

IN AREAS WHERE THE DRAWINGS DO NOT ADDRESS METHODOLOGY, THE CONTRACTOR SHALL BE BOUND TO PERFORM IN STRICT COMPLIANCE WITH MANUFACTURER'S SPECIFICATIONS AND/OR RECOMMENDATIONS.

ON-SITE VERIFICATION OF ALL DIMENSIONS AND CONDITIONS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND HIS SUBCONTRACTORS. NOTED DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE.

THE GENERAL NOTES AND TYPICAL DETAILS APPLY THROUGHOUT THE JOB UNLESS OTHERWISE NOTED OR SHOWN.

THE CONTRACTOR SHALL COMPARE AND COORDINATE ALL DRAWINGS. IF A DISCREPANCY EXISTS, HE SHALL PROMPTLY REPORT IT FOR PROPER ADJUSTMENT BEFORE PROCEEDING WITH THE WORK. IN THE EVENT THAT CERTAIN FEATURES OF THE CONSTRUCTION ARE NOT FULLY SHOWN ON THE

DRAWINGS, THEIR CONSTRUCTION SHALL BE OF THE SAME CHARACTER AS SIMILAR CONDITIONS THA ARE SHOWN OR NOTED. THESE PLANS ARE SUBJECT TO MODIFICATIONS AS NECESSARY TO MEET CODE REQUIREMENTS OR TO

FACILITATE MECHANICAL, PLUMBING INSTALLATIONS OR TO INCORPORATE DESIGN IMPROVEMENTS. DO NOT BUILD OVER GAS LINES OR ENCLOSE THE METER. CONSULT THE LOCAL GAS COMPANY PRIOR

CHIMNEY SHALL EXTEND AT LEAST 2 FEET HIGHER THAN ANY PORTION OF THE BUILDING WITHIN 10 FEET, BUT SHALL NOT BE LESS THAN 3 FEET ABOVE THE POINT WHERE IT PASSES THROUGH THE

DECKS ARE NOT APPROVED FOR FUTURE HOT TUB INSTALLATION.

NO OPENING NOR ANY CHANGES IN SIZE, DIMENSION OR LOCATION SHALL BE MADE IN ANY STRUCTURAL ELEMENTS WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.

CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO ORDERING MATERIALS OR PROCEEDING WITH NEW WORK IN AREAS AFFECTED BY EXISTING CONDITIONS. STRUCTURAL ENGINEER SHALL BE INFORMED IN WRITING OF CONFLICTS BETWEEN EXISTING AND PROPOSED NEW CONSTRUCTION.

CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL DIMENSIONS SHOWN ON THE CONTRACT DOCUMENTS. INCONSISTENCIES ON THE STRUCTURAL DRAWINGS OR BETWEEN THE STRUCTURAL DRAWINGS AND ANY OTHER CONTRACT, SHOP, FABRICATION, OR OTHER DRAWINGS OR INFORMATION SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH AFFECTED WORK.

THE STRUCTURAL INTEGRITY OF THE BUILDING IS DEPENDANT UPON COMPLETION ACCORDING TO PLANS AND SPECIFICATIONS. THE STRUCTURAL ENGINEER ASSUMES NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION. THE METHOD OF CONSTRUCTION AND SEQUENCE OF OPERATIONS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL SUPPLY ANY NECESSARY SHORING, BRACING, GUYS, ETC., TO PROPERLY BRACE THE STRUCTURE AGAINST WIND, DEAD AND LIVE LOADS UNTIL THE BUILDING IS COMPLETED ACCORDING TO THE PLANS AND SPECIFICATIONS.

CONTRACTOR SHALL NOT PLACE BACK FILL AGAINST BASEMENT WALLS UNTIL THE FLOOR SYSTEM IS COMPLETELY INSTALLED OR CONTRACTOR HAS PROVIDED ADEQUATE SHORING AND BRACING. ANY QUESTIONS REGARDING TEMPORARY SHORING REQUIREMENTS SHOULD BE FORWARDED TO THE STRUCTURAL ENGINEER FOR REVIEW

### FOUNDATION WALL STRIP FOOTING SCHEDULE 8" CONCRETE WALL (D) FOOTING SIZES SOIL CLASSIFICATIONS / PRESUMPTIVE BEARING CAPACITIES SHEAR -D KEY -\ i MH & CH | SC, GM & GC (1500 psf) (2000 psf) (2500 psf) (3000 psf) MIN MARK SIZE (TxW) SIZE (TxW) | SIZE (TxW) | SIZE (TxW) 8" x 16" W8B 8" x 18" 8" x 16" **. .** . W8C W8D 8" x 18" ₩*─* W8E 3" CLR COVER -W8F 8" x 20" #4 @ 12"oc w/#3 8" x 22" DOWELS @ 48"oc ---M8H 8" x 24" • CONCRETE COMPRESSIVE STRENGTH = 3,000 psi L8M 12" x 34" \ 10" x 26" • REBAR = GRADE 60 14" x 36" 10" x 28" DEFORMED 10" CONCRETE WALL (D) FOOTING SIZES SOIL CLASSIFICATIONS / PRESUMPTIVE BEARING CAPACITIES | SC, GM & GO (1500 psf) (2000 psf) (2500 psf) | (3000 psf) SIZE (TxW) SIZE (TxW) | SIZE (TxW) | SIZE (TxW) WI0B 8" x 18" MI0C 8" x 20" WI0D WI0E WIOF | 8" x 20" THICKENED SLABS WIOG 8" x 22" EQUIVALENT MIOH WALL FOOTING 8" x 24" MIOI TSI6 W8C MIO.I 8" x 26" TSI8 W8D 14" x 36" 10" x 28" WIOK L TS20 W8E T522 W8F 12" CONCRETE WALL (D) FOOTING SIZES TS24 W8G SOIL CLASSIFICATIONS / PRESUMPTIVE BEARING CAPACITIES MH & CH | SC, GM & GC GW, GP (1500 psf) | (2000 psf) | (2500 psf) | (3000 psf) MARK | SIZE (TxW) | SIZE (TxW) | SIZE (TxW) | SIZE (TxW) 8" x 20" WI2D 8" x 20" WI2E WI2F WI2G 8" x 22" WI2I

ISOLATED FOOTING SCHEDULE

SQUARE, ISOLATED FOOTING SPECIFICATIONS

FA | 30"XI2" -0 | 24"XI2" -0 | 24"XI2" -0 | 24"XI2" -0 |

FB | 32"XI2" -0 | 26"XI2" -0 | 24"XI2" -0 | 24"XI2" -0 |

FC | 34"X12" -0 | 28"X12" -0 | 24"X12" -0 | 24"X12" -0

FG | 44"X12" -3 | 36"X12" -0 | 32"X12" -0 | 30"X12" -

FJ | 50"XI2" -3 | 42"XI2" -3 | 38"XI2" -2 | 34"XI2" -:

FK 52"XI2" -3 44"XI2" -3 40"XI2" -2 36"XI2"

FL | 54"XI2" -3 | 46"XI2" -3 | 42"XI2" -3 | 38"XI2" -2

FM | 58"XI2" -4 | 48"XI2" -3 | 44"XI2" -3 | 40"XI2" -2

FN | 60"X12" -5 | 50"X12" -4 | 46"X12" -4 | 42"X12" -3

FP | 64"XI2" -6 | 54"XI2" -4 | 48"XI2" -4 | 44"XI2" -4

FQ | 66"X12" -6 | 56"X12" -5 | 50"X12" -5 | 46"X12" -4

FR | 68"XI2" -7 | 58"XI2" -6 | 52"XI2" -5 | 48"XI2" -5

FT | 74"XI2" -9 | 62"XI2" -7 | 56"XI2" -6 | 50"XI2" -5

FU | 76"XI2" -I0 | 64"XI2" -8 | 58"XI2" -7 | 52"XI2" -6

FV | 78"XI4" -I0 | 66"XI2" -8 | 60"XI4" -8 | 54"XI2" -7

FW | 80"X14" -11 | 68"X14" -9 | 62"X14" -8 | 56"X14" -

FX | 82"X14" -11 | 70"X14" -9 | 62"X14" -8 | 58"X14" -8

FY | 86"XI4" -12 | 72"XI4" -10 | 64"XI4" -9 | 58"XI4" -8

FZ 88"XI4" -I3 74"XI4" -II 66"XI4" -9 60"XI4" -8

N = NUMBER OF #4 BOTTOM BARS, EACH WAY

FS 72"X12" -8 60"X12" -6 54"X12" -6 50"X12" -

FO 62"X12" -5 52"X12" -4 48"X12" -4 42"X12"

FH | 46"X12" -3 | 38"X12" -2 | 34"X12" -2 | 32"X12" -2

FI | 48"XI2" -3 | 40"XI2" -2 | 36"XI2" -2 | 34"XI2" -2 |

CL, ML,

MH & CH | SC, GM & GC

SOIL CLASSIFICATIONS/PRESUMPTIVE

(1500psf) | (2000psf) | (2500psf) | (3000psf)

EQUIVALENT FOOTINGS

BEARING CAPACITIES

8" x 26"

SOIL CLASSIFICATIONS

CH (1500psf) SW, SP, SM,

- FLEXURE REINFORCEMENT, BOTH DIRECTIONS

PER FOOTING SCHEDULE, MIN. 2" \$ 3" CLEAR

FROM FOOTING SIDE & BOTTOM, RESPECTIVELY

QUARE FOOTING, TYPICAL SECTION

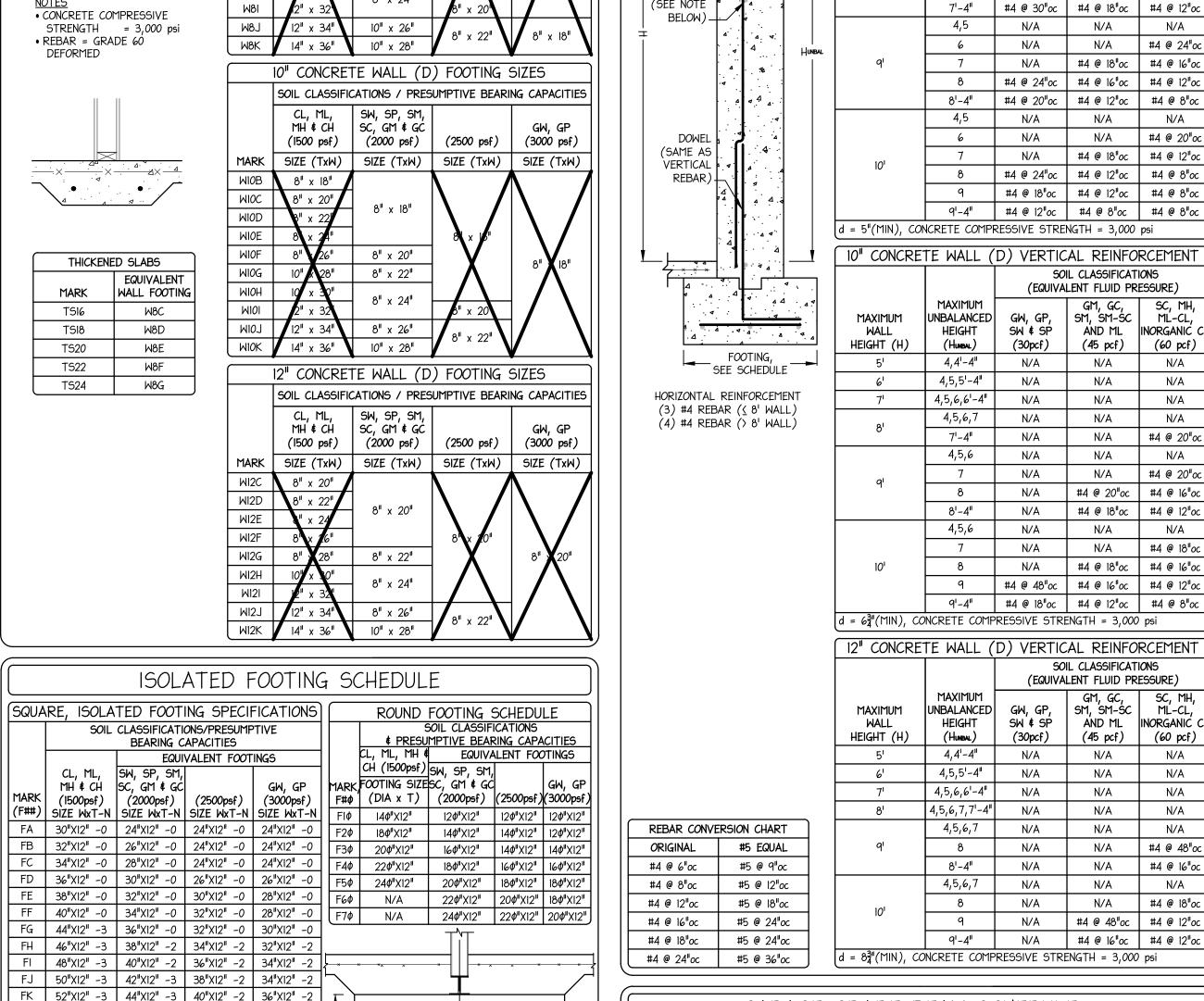
11-12

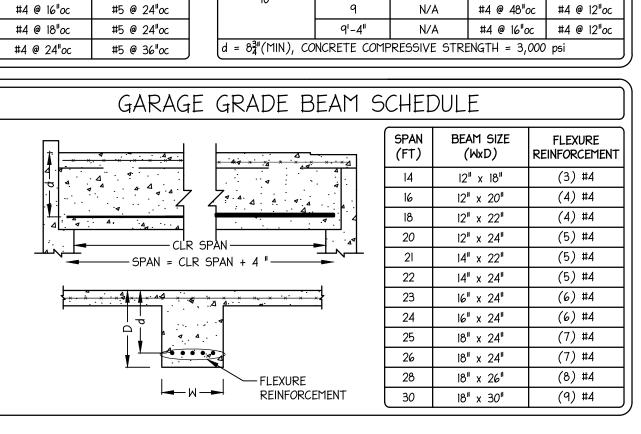
REINFORCEMENT EQUIVALENTS

BARS SPEC'D BARS REQ'D

NUMBER #4 NUMBER #5

MARK FOOTING SIZESC, GM & GC





3929 BALTIMORE ST. KENSINGTON MD 20895

GTM ARCHITECTS ARCHITECT

CONTRACTOR

PROFESSIONAL CERTIFICATION. I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL

ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND.

LICENSE NO.: 24518

EXPIRATION DATE: 09-21-2021



SCALE: AS NOTED		
DRAWN BY: 5L	CHECKED BY: JMJ	
ISSUE:	DATE:	
ISSUED FOR PERMITS	04-27-2020	
REVISION:		
1 FRAMING REVISION	10-29-2020	
2		
3		
2 3 4 5 6 7		
5		
6		
8		
-		

GENERAL STRUCTURAL NOTES & SCHEDULES

