

HISTORIC PRESERVATION COMMISSION

Marc Elrich County Executive Sandra I. Heiler Chairman

Date: April 2, 2020

MEMORANDUM

TO:	Hadi Mansouri
	Department of Permitting Services
FROM:	Michael Kyne
	Historic Preservation Section
	Maryland-National Capital Park & Planning Commission
SUBJECT:	Historic Area Work Permit #904304: Demolition and stabilization

The Montgomery County Historic Preservation Commission (HPC) has reviewed the attached application for a Historic Area Work Permit (HAWP). This application was <u>Approved</u> at the March 25, 2020 HPC meeting.

The HPC staff has reviewed and stamped the attached construction drawings.

THE BUILDING PERMIT FOR THIS PROJECT SHALL BE ISSUED CONDITIONAL UPON ADHERENCE TO THE ABOVE APPROVED HAWP CONDITIONS AND MAY REQUIRE APPROVAL BY DPS OR ANOTHER LOCAL OFFICE BEFORE WORK CAN BEGIN.

Applicant:First Agape AME Zion Church (Thomas Taltavull, Architect)Address:7700 Seven Locks Road, Bethesda

This HAWP approval is subject to the general condition that the applicant will obtain all other applicable Montgomery County or local government agency permits. After the issuance of these permits, the applicant must contact this Historic Preservation Office if any changes to the approved plan are made. Once work is complete the applicant will contact Michael Kyne at 301.563.3403 or michael.kyne@montgomeryplanning.org to schedule a follow-up site visit.













REVIEWED

By Michael Kyne at 1:36 pm, Apr 02, 2020







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By Michael Kyne at 1:36 pm, Apr 02, 2020



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DEMOLITION FLOOR PLAN

ate: Feb. 3, 2020

Building Demoktiion: FIRST AGAPE AME ZION CHURCH 7700 Seven Locks Road Bethesda, Maryland 20817

Drawing Numbe A1.0

REMOVE EXISTING NON CONTRIBUTING WOOD STEPS.

DEMOLITION PLAN NOTES:

- (2) REMOVE EXISTING NON CONTRIBUTING DOUBLE DOORS. HEAD TRIM ABOVE IS CONTRIBUTING AND TO REMAIN.
- EXISTING DOUBLE WOOD HALF LIGHT DOORS AND TRIM TO REMAIN.
- (4) EXISTING CLOSET AND DOOR TO REMAIN.
- (5) EXISTING DOUBLE HUNG WOOD WINDOWS, SIX OVER ONE. CONTRACTOR TO REMOVE AND SALVAGE ALONG WITH INTERIOR AND EXTERIOR TRIM, TYPICAL OF SEVEN.
- (6) EXISTING BAY WINDOW TO BE REMOVED
- T EXISTING BRICK CHIMNEY TO BE REMOVED
- TYPICAL CHURCH WALL CONSISTS OF GYPSUM DRYWALL, WOOD BASE, 2 x 4 WOOD STUDS @ 16" or, 1 x 6 SHEATHING BOARDS, 5" WOOD DROP SIDING AND 11" ASBESTOS SIDING, REMOVE ASBESTOS SIDE SALVAGE WOOD SIDING AND STORE AND PROTECT FROM WEATHER FOR REUSE.
- (9) REMOVE WOOD FLOOR FRAMING (2×12" @ 167).
- (1) REMOVE WOOD ROOF FAMING 2 x 6 @ 24" oc W 2 x 6 @ 24 CEILING JOISTS AT 11'-2" A.F.F.
- (1) REMOVE ROOF AND FLOOR FRAMING IN CHURCH DAMAGED BY FALLEN TREE AND WATER INTRUSION.
- (2) REMOVE CARPET FLOORING
- (3) REMOVE CONCRETE MASONRY UNIT EXTERIOR WALLS AND FOUNDATIONS AT CHURCH
- (1) REMOVE ALL WOOD ROOF FRAMING AT 1979 ADDITIONS COMPLETELY DAMAGED BY FALLEN TREE AND WATER INFILTRATION
- (5) REMOVE ASSUMED WOOD FLOOR FRAMING OVER CRAWAL SPACE AND CMU FOUNDATION WALLS AND FOOTINGS.
- (b) ASSUMED CONCRETE SLAB ON GRADE, CMU FOUNDATION WALL AND FOOTINGS TO REMAIN UNTIL FUTURE ADDITION IS CONSTRUCTED.
- 17 REMOVE PORCH, STEPS AND RAILINGS.
- (B) CONTRACTOR TO VERIFY LOCATION OF OIL TANK IN CRAVIL SPACE AND REMOVE.
- (19) EXISTING ELECTRICAL PANEL, METER AND OVERHEAD SERVICE LINE TO BE REMOVED.
- PLUMBING, MECHANICAL AND ELECTRICAL CONTRACTOR TO VISIT SITE AND CONSULT WITH OWNER CONCERNING ANY SYSTEMS AND EQUIPMENT TO REMAIN, ALL OTHER EQUIPMENT, DEVICES, PRIME ETC SHALL BE REMOVED. 20





DURST & TAYLOR

Structural Engineering, LLC

David G. Durst, P.E. 1228 Copper Beech Drive, York, PA 17403 Cell: (717) 793-7723 Email: <u>ddurst@dtsellc.com</u>

Nevin E. Taylor, P.E. 211 Glenview Road, Spring Grove, PA 17362 Cell: (717) 515-1590 Email: <u>ntaylor@dtsellc.com</u>

April 19, 2018

Elmer Anderson, Project Manager Holland Construction 751 Frederick Street Hanover, PA 17331 **REVIEWED** By Michael Kyne at 1:34 pm, Apr 02, 2020

Re: Structural Assessment Letter Report First Agape AME Zion Church 7700 Seven Locks Road, Bethesda, MD 20817 DTSE Project No. 0054-003-01 APPROVED Montgomery County Historic Preservation Commission Aandrad . Heiler

Dear Mr. Anderson:

Per your request, Durst & Taylor Structural Engineering, LLC (DTSE) performed a site visit to the above-noted property on April 17, 2018 for the purposes of determining the existing structural conditions as well as to assess the structure for possible rehabilitation. This letter summarizes the findings of this site visit, including a brief description of the structure, inspection approach and findings, as well as our conclusions and recommendations.

Brief Structure Description

The existing structure is believed to be well over 100 years old, having been previously renovated in 1923 (as noted on the cornerstone at the northeast corner). It is situated on a wooded hillside just northwest of the location where Seven Locks Road extends beneath Interstate 495 (see Photo Nos. 1 thru 3). As there is no on-site parking, access is made from a small parking lot on the east side of Seven Locks Road and just north of the structure. As seen in Photo Nos. 4 thru 10, this one-story, wood-framed structure is clad with asbestos wall tiles and asphalt roof shingles. It is believed that the original structure consists of the gabled roof portion along the north side (including the bell tower near the front of the roof), with prior additions to the south. Foundations primarily consist of concrete masonry unit (i.e., CMU, or "concrete block") walls believed to be sitting on shallow spread concrete footings.

Inspection Approach

Access to the locked and boarded structure was provided by Holland Construction. The vast majority of our inspection involved visual observations of the building and its structural components. Beyond this, only a few random hammer soundings of the foundation walls were performed to determine the latter's general condition. Documentation was made via written notes and digital photographs. Tools included a flashlight, awl (wood penetration testing), hammer, and tape measure (to determine general member sizes and spacing). No material sampling or testing was performed as part of this assignment.

Structural Assessment Letter Report First Agape AME Zion Church, Bethesda, MD DTSE Project No. 0054-003-01

Inspection Findings and Conclusions

In 2007, the existing structure was damaged by a fire which caused it to be uninhabitable since that time. Approximately four years ago, the structure was impacted by a collapsed tree which severely damaged the south and west portions of the roof structure such that there has been a very large opening in the roof (Photo Nos. 8 and 10 thru 13). Not only has this portion of the roof been completely compromised, the structure beneath this opening has been exposed to the elements for over 10 years, including rain, wind, snow, and frankly, local wildlife. In our opinion, this portion of the structure cannot be saved.

Though no area of the structure is completely unaffected by the fire or the collapsed tree, the front (east end) of the structure is relatively unscathed in comparison to the west and south portions. As seen in Photo Nos. 14 thru 16, the roof and wall framing of the entrance enclosure as well as the first 5' to 6' of the main structure (including the bell tower) remain upright, plumb and intact. In our opinion, it is indeed possible to carefully detach this portion of the structure from the remainder of the structure for the purposes of re-integrating it into a replacement structure.

Recommendations

Although it is possible to save the small front portion of the structure that was generally unaffected by the fire or tree collapse, we would be remiss if we did not mention that doing so would entail time-consuming and costly shoring to temporarily stabilize and remove such from its existing location so that the remainder of the structure can be demolished. It should also be noted that the lifting and transport of this portion of the structure will involve risk inherent with such activities. For these reasons, our primary recommendation is that the entire structure should be demolished and replaced in-kind.

Assuming that complete demolition is deemed to be unacceptable, we strongly recommend that only the entrance enclosure and the first 5' to 6' of the main structure (including the bell tower) be saved, and that the remainder of the structured be demolished. In addition, the exterior cladding of the portion to be saved (i.e., the asbestos wall tiles and severely compromised asphalt roof shingles) should be removed in its entirety due to health and material degradation reasons. In other words, only the underlying wood framing elements should be re-integrated into the replacement structure.

If you have any questions regarding the content of this letter, or if you require additional services, please do not hesitate to contact us.

_	Sincerely,						
F	REVIEWED	DURST	& TAYLOR STRUCTURAL ENGINEERING, LLC				
L	By Michael Kyne at 1:34 pm, Apr 02, 2020						
	APPROVED	Nevin E.	Taylor, P.E., Partner/Structural Engineer				
	Montgomery County	Maryland	d P.E. No. 21386 (exp. 10-26-2018)				
	Historic Preservation Commission						
Attachments							
Cc:	Filandral. Keiler D. Durst						



Photo No. 1: Aerial view of 7700 Seven Locks Road, Bethesda, Maryland. Note that the church structure is located in the northwest quadrant of where Seven Locks Road runs beneath Interstate 495.



Photo No. 2: View of church structure (looking southwest) as seen from the parking lot just north and on the other side of Seven Locks Road.



Photo No. 3: Similar (closer) view as previous photo.



Photo No. 4: Front (east) elevation view of structure as seen from the steps leading up to the entrance from Seven Locks Road.



Photo No. 5: View of the front entrance as seen from the northeast corner. Note that the structure is wood-framed and clad with painted asbestos wall tiles and asphalt roof shingles.



Photo No. 6: Partial view of the front half of the north elevation showing boarded wall openings and windows, extreme moss growth on the intact portion of the roof, as well as the collapsed section of the roof (right side).



Photo No. 7: View of the northwest corner of the structure, showing similar issues noted in the previous photo.



Photo No. 8: View of the southwest portion of the structure (see from the northwest) which has been impacted by a collapsed tree (purportedly in 2007).



Photo No. 9: General view of the structure's southeast corner as seen from the level of the streambed to the south. Note that the roof of the structure's southwest has been completely collapsed; however, the front portion of the structure (including front entranceway and bell tower) remain intact.



Photo No. 10: Close-up view of the bell tower.



Photo No. 11: Interior view of the structure (looking west from the front entranceway), exhibiting the collapsed roof of the southwest corner.



Photo No. 12: Similar view as the previous photo, looking south from the center of the floor plan.



Photo No. 13: View collapsed roof framing along the west end of the structure. Note that wall and roof framing consists of 1" thick wood boards of various widths attached to solid sawn rafters or studs.



Photo No. 14: View of structure's relatively intact northeast corner at the front entrance area.



Photo No. 15: View of structure's relatively intact southeast corner at the front entrance area.



Photo No. 16: View of structure's intact roof framing above the front entrance area.

STRUCTURAL SPECIFICATIONS AND GENERAL CONDITIONS GENERAL

- . Where these specifications conflict with other project specifications, these specifications shall govern. . Durst & Taylor Structural Engineering, LLC will assume no responsibility and/or liability for problems which arise from failure to follow these plans, specifications, and the design intent they convey or for problems which arise from
- others' failure to obtain and/or follow the Engineer's guidance. . All work shall be performed in accordance with local applicable codes and regulations. Appropriate safety
- measures satisfying local and OSHA requirements shall be provided.
- 4. Proper temporary bracing of all construction work in progress is the Contractor's responsibility. 5. If during demolition existing conditions do not agree with information shown on the design drawings, the Contractor
- shall notify the Engineer immediately. 6. The Contractor shall be responsible for the location and protection of all existing utilities during construction and the repair of any damaged facilities.
- Sections and details shown, while drawn for specific locations, are intended to establish the general types of details
- to be used throughout.
- 8. Drawings should not be scaled. Contact the Engineer for clarification of any dimension in question. 9. All dimensions shall be verified by the Contractor. Layout shall be checked and coordinated between all construction documents and specifications prior to the start of work.
- DESIGN DATA
- 1. Building Code = 2015 International Building Code
- 2. Floor Load: 2.1. Dead Load = 15 PSF (includes structure weight)
- 2.2. Floor Live Loads = 40 PSF (for purposes of temporary stabilization)
- Roof Load:
- 3.1. Dead Load = 15 PSF (includes structure weight)
- 3.2. Live Load = 30 PSF
- 4. Snow Load: (Roof Live Load May Control)
- 4.1. Ground Snow Load, Pg = 30 PSF
- 4.2. Flat Roof Snow Load, Pf = 24.5 PSF 4.3. Snow Exposure Factor, Ce = 1.0
- 4.4. Snow Load Importance Factor, Is = 1.0
- 4.5. Thermal Factor. Ct = 1.0
- 5. Wind Load:
- 5.1. Basic Wind Speed (3-second gust) = 115 MPH
- 5.2. Wind Importance Factor, Iw = 1.0
- 5.3. Building Category = II 5.4. Exposure Category = C
- 5.5. Internal Pressure Coefficient, GCpi = ± 0.18
- 5.6. Wind Design Pressure (MWFRS) = 25 PSF
- 6. Earthquake Design: Seismic Design Load Cases and Combination do not govern over Wind
- 7. Concrete Design Method:
- 7.1. Design per ASD 7.2. Loads indicated are ASD loads
- 8. Wood Framing Design Method:
- 8.1. Design per ASD
- 8.2. Loads indicated are ASD loads
- SPECIAL INSPECTION REQUIREMENTS
- . The following types of work require special inspection based on Section 1704 of the 2012 International Building Code. The owner will employ special inspectors who shall provide special inspections for compliance with the construction documents and other references noted. Reports shall be submitted to the Engineer and Building Official on a periodic basis. A final report shall be submitted documenting required special inspections and correction of any discrepancies prior to the end of construction. 1.1. Soils
- 1.1.1. Verify materials below footings are adequate to achieve the design bearing capacity.
- 1.1.2. Verify excavations are extended to proper depth and have reached proper material.
- Concrete Construction
- 2.1. Periodic inspection of reinforcing steel for compliance with approved Construction Documents and ACI 318, Sections 3.5 and 71-77
- 2.2. Periodically verify the use of required concrete design mixtures, in accordance with ACI 318, Chapter 4 and Section 5.2 - 5.4.
- 2.3. Verify sampling of fresh concrete to determine slump, air content and temperature when making specimens for strength tests, in accordance with ACI 318, Sections 5.6 and 5.8. 2.4. Periodic inspection for maintenance of curing temperatures and techniques to ensure compliance with ACI 318, Sections 5.11 -
- 5.13.
- 3. Wood Construction
- 3.1. Inspect wood structural panel sheathing for diaphragms to ensure correct grade and thickness.
- 3.2. Verify nominal size of framing members at adjoining panel edges.
- 3.3. Verify fastener diameter and length, number of fastener lines, and spacing between fasteners in each line and at edge margins.

- CONSTRUCTION PROCEDURES AND SAFETY REQUIREMENTS
- 1. The contract structural drawings and specifications represent the finished structure. Unless otherwise indicated, they do not indicate the means or methods of construction.
- 2. Provide all measures necessary to protect the workmen and other persons during construction. Provide all necessary measures to avoid excessive stresses and to hold the structural elements in place during construction. Such measures shall include, but not be limited to, bracing, shoring for construction equipment, shoring for earth
- banks, forms, scaffolding, planking, safety nets, support and bracing for cranes and hoists, guying, etc.
- 3. Engage properly qualified persons to determine where and how temporary precautionary measures shall be used. Observation visits to the site by structural engineer's field representative shall not include the items noted above.
- 4. Supervise and direct the work so as to maintain sole responsibility for all construction means, methods, techniques, sequences, and procedures. Retain the services of a professional structural engineer licensed in the state in which the project is located to design and supervise any scaffolding for workmen, and all shoring of forms and elements of the construction.

CAST-IN-PLACE CONCRETE CONSTRUCTION

TYPE OF CONSTRUCTION	MIN. 28-DAY	Max.	TYPE 1
	COMP.	W/C	PORTLAND
	STRENGTH	Ratio	CEMENT
(1) Fill Concrete for Wood Posts	3000 PSI	.53	5 BAGS

- 1. All concrete work shall conform to the requirements of ACI 318, Building Code Requirements for Reinforced Concrete, latest edition and ACI 301, Specifications for Structural Concrete for Buildings, latest edition, including all revisions, except as modified herein.
- 2. Concrete shall be supplied by a qualified ready-mixed concrete plant in accordance with the following requirements: 2.1. Entrained Air = 4 to 7%
- 2.2. Coarse aggregate shall be AASHTO NO. 57 stone. Maximum aggregate size = 1.5"
- 2.3. Submit mix designs to Engineer for approval. No admixtures permitted without Engineer's approval. 2.4. Chloride containing admixtures are not permitted.
- 2.5. Fine aggregate must be natural sand, unless approved by the Engineer.
- 3. When concrete arrives at the project with slump below that suitable for placement, as indicated by the Specifications, water may be added only if neither the maximum permissible water-cement ratio nor the maximum slump is exceeded. The water shall be incorporated by additional mixing equal to at least half of the total mixing required. Discharge of the concrete shall be completed within 1-1/2 hours, or before the truck drum has revolved 300 revolutions, whichever comes first, after the introduction of the mixing water to the cement and aggregates or the introduction of the cement to the aggregates. Truck batch slips must include time of batching, total drum revolutions upon arrival at site, and quantity of water (in gallons) per cubic yard available to be added to attain the maximum design water-cement ratio.
- 4. Concrete shall be placed only after approval of the reinforcement and mix designs by the acting special inspection agency or the Engineer. Contractor is responsible for coordinating inspections prior to concrete placement.
- 5. Schedule the pouring of foundations on the same day the excavation is completed. 6. Cure all concrete in accordance with accepted ACI Standards and conformance to ACI 308.1, Standard Specification for Curing Concrete, latest edition, for the worst case weather conditions anticipated during the curing period. All concrete construction and procedures shall conform to the requirements of ACI 306.1, Standard Specification for Cold Weather Concreting, latest edition.
- 7. No aluminum of any type shall be allowed in the concrete work unless coated to prevent aluminum-concrete reaction. 8. Mechanically vibrate concrete.
- 9. Do not place pipes, ducts, reglets or chases in structural concrete or composite floor systems without approval of the structural engineer.
- 10. Maximum free drop of any concrete = 5'-0".
- 11. Six (6) test cylinders shall be prepared for every 50 cubic yards of concrete placed on any given day to represent all concrete placed on that day. Two (2) cylinders shall be tested at seven (7) days and two (2) cylinders shall be tested at 28 days for verification of concrete design strength with two (2) cylinders remaining.
- 12. Concrete submittals required to be reviewed and approved by the Engineer prior to cast-in-place concrete construction include, but are not limited to: Mix designs.



SLUMP RANGE	
3" - 6"	

WOOD FRAMING

- 1. All work shall be performed in accordance with the National Design Specification for Wood Construction, latest edition, as issued by the AFPA.
- 2. All materials, unless noted otherwise, shall be as follows:
- 2.1. All exposed wood and wood in direct permanent contact with masonry or concrete shall be pressure-treated to a minimum 0.4 PCF. All pressure treating shall be free of arsenic.
- 2.2. Studs, plates, ledgers, and bracings = Southern pine Construction Grade.
- 2.3. Joists and beams = Southern pine No. 2 or better.
- 2.4. Posts = Southern pine No. 2 or better.
- 2.5. Moisture content of all structural lumber shall be 19% maximum as verified by stamp. 2.6. All hardware for pressure-treated lumber, fire-retardant-treated lumber, or other exterior lumber shall be stainless steel (grade 304 or better).
- Wall sheathing shall be nailed to framing members with 8d nails according to the following patterns:
- 3.1. At supported edges of each sheathing panel, nails shall be spaced at 6" o.c. max., U.N.O. 3.2. At intermediate supports of each sheathing panel, nails shall be spaced at 12" o.c. max., U.N.O.
- 3.3. Sheathing panels for shear walls shall not be less than 4'x8', except at boundaries and changes in framing, and framing members or blocking shall be provided at the edges of all panels.

REVIEWED By Michael Kyne at 1:36 pm, Apr 02, 2020





NOTES: 1. ALL EXISTING GRADES BOTH INTERIOR AND EXTERIOR ARE ASSUMED. CONTRACTOR SHALL VERIFY IN FIELD. 2. CONTRACTOR IS RESPONSIBLE TO SEAL THE STRUCTURE AGAINST WEATHER, ETC.

SCALE: 1/4"=1'-0





March 11, 2020

To: Historic Preservation Commission (HPC) Maryland-National Capital Park and Planning Commission

From: Thomas Taltavull

Re: HAWP for First Agape AME Zion Church Master Plan Site #29/39 Addenda to HAWP

Addenda A

Based on preliminary consultation comments from the HPC we offer the following additional information:

- 1. The method of documenting the building prior to demolition will include photographs of all exterior elevations and interior views. Measured drawings of the church have been included in the application prepared from architect's field measurements.
- 2. The owner has provided a stabilization plan for the front portion of the church that will remain. The plan was prepared by the structural engineering firm Durst and Taylor. The west elevation of the remaining stabilized section is indicated to be sheathed in plywood and will be weatherized with an air and water infiltration barrier building wrap, (Tyvek). The contractor will ensure that the church will be protected from the weather and secured.
- 3. All salvageable materials including wood siding, trim, windows will be stored in the stabilized section of the church to remain. If required the Owner has indicated that they will obtain a metal storage container if additional space is needed.

Please let me know if you need any additional information or have any questions.



APPROVED Montgomery County Historic Preservation Commission fandral. Heiler

20650 Plum Creek Court, Laytonsville, Maryland 20882 301.840.1847